# Bay Mud

***FIG_18:  Title***

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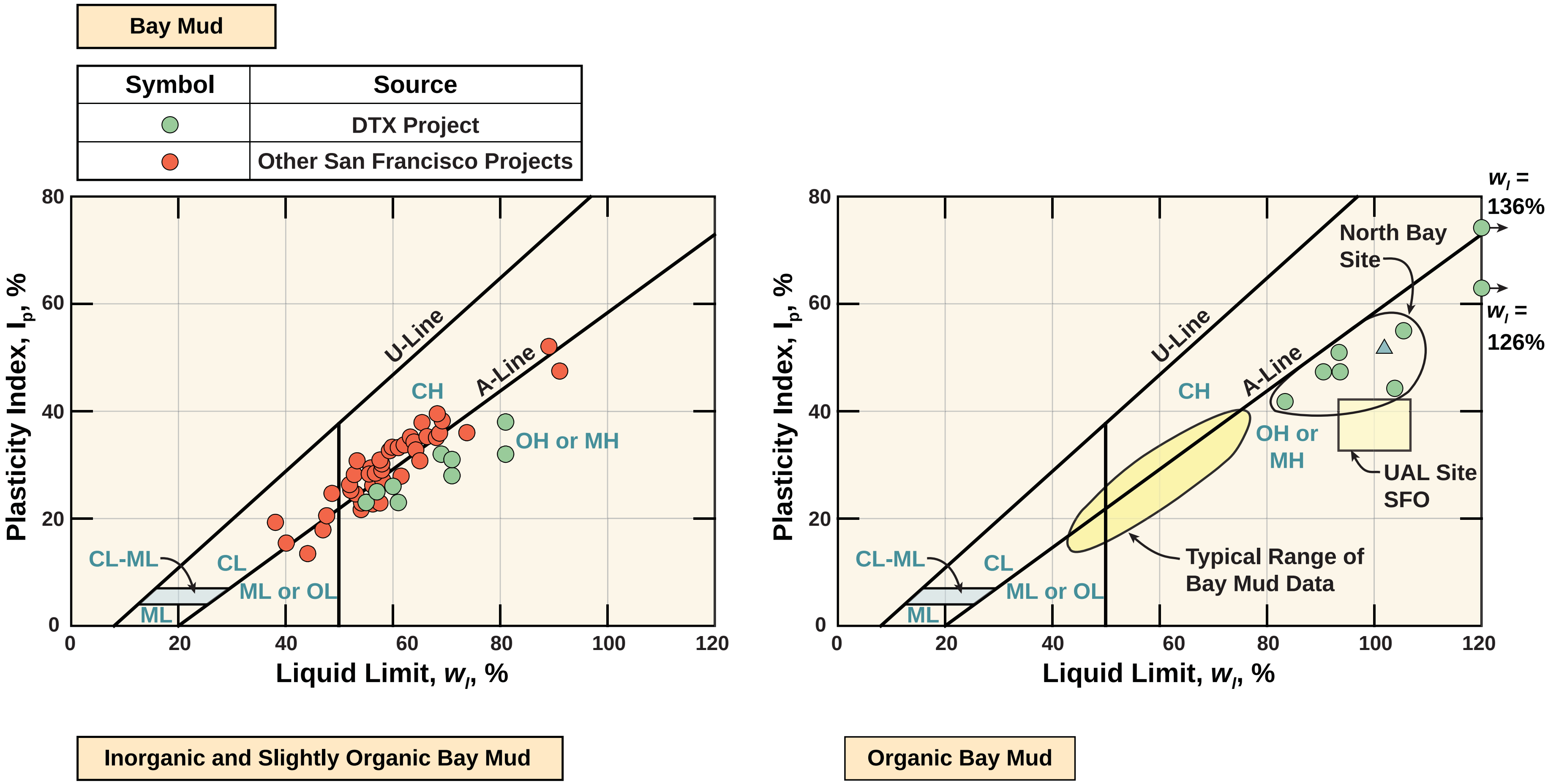


*FIG_18A: Atterberg Limits of Bay Mud*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_18A*

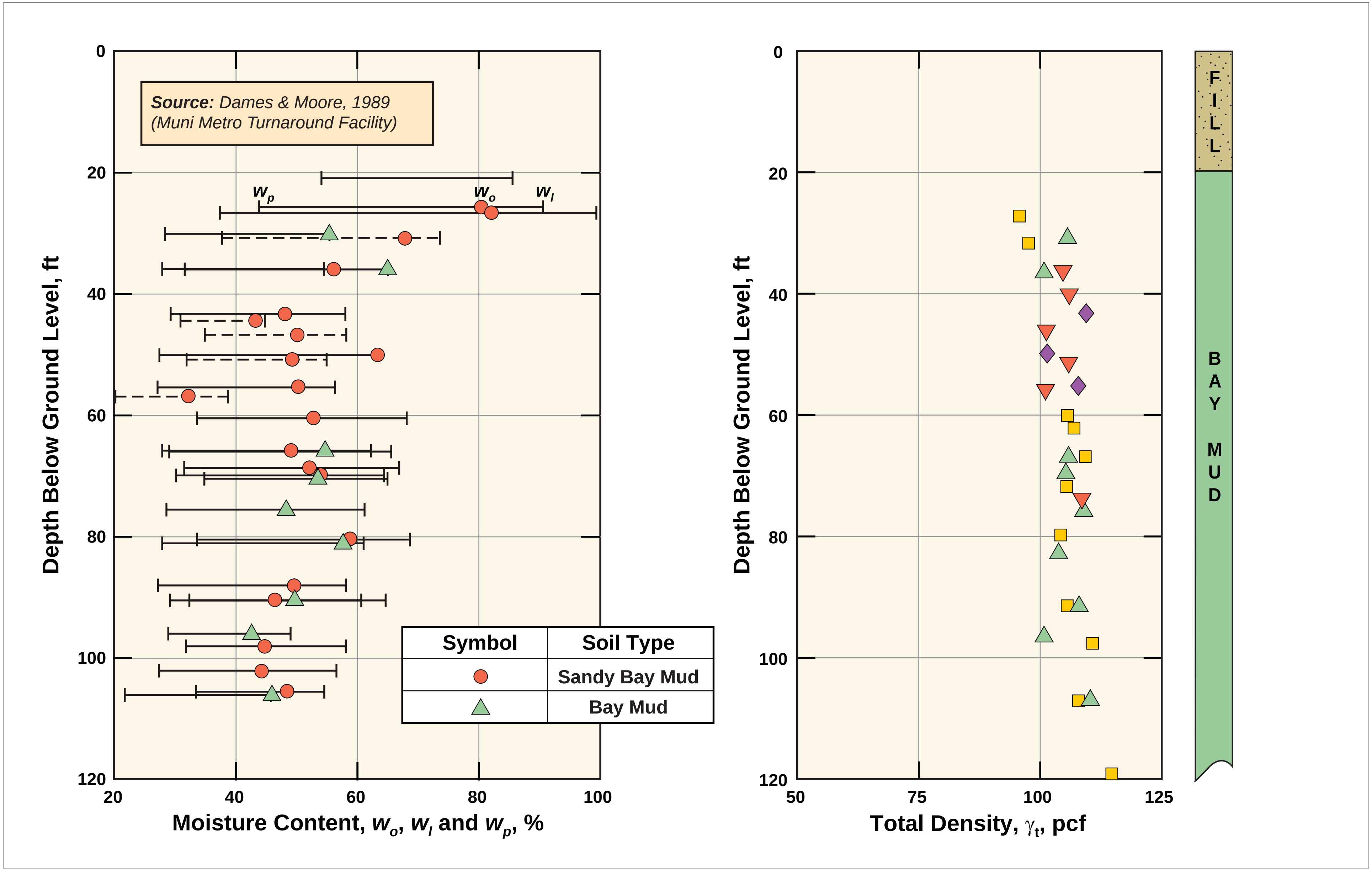


*FIG_19: IVariation of Index Properties with Depth Muni Metro Turnback Site (Embarcadero Waterfront)*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_19*

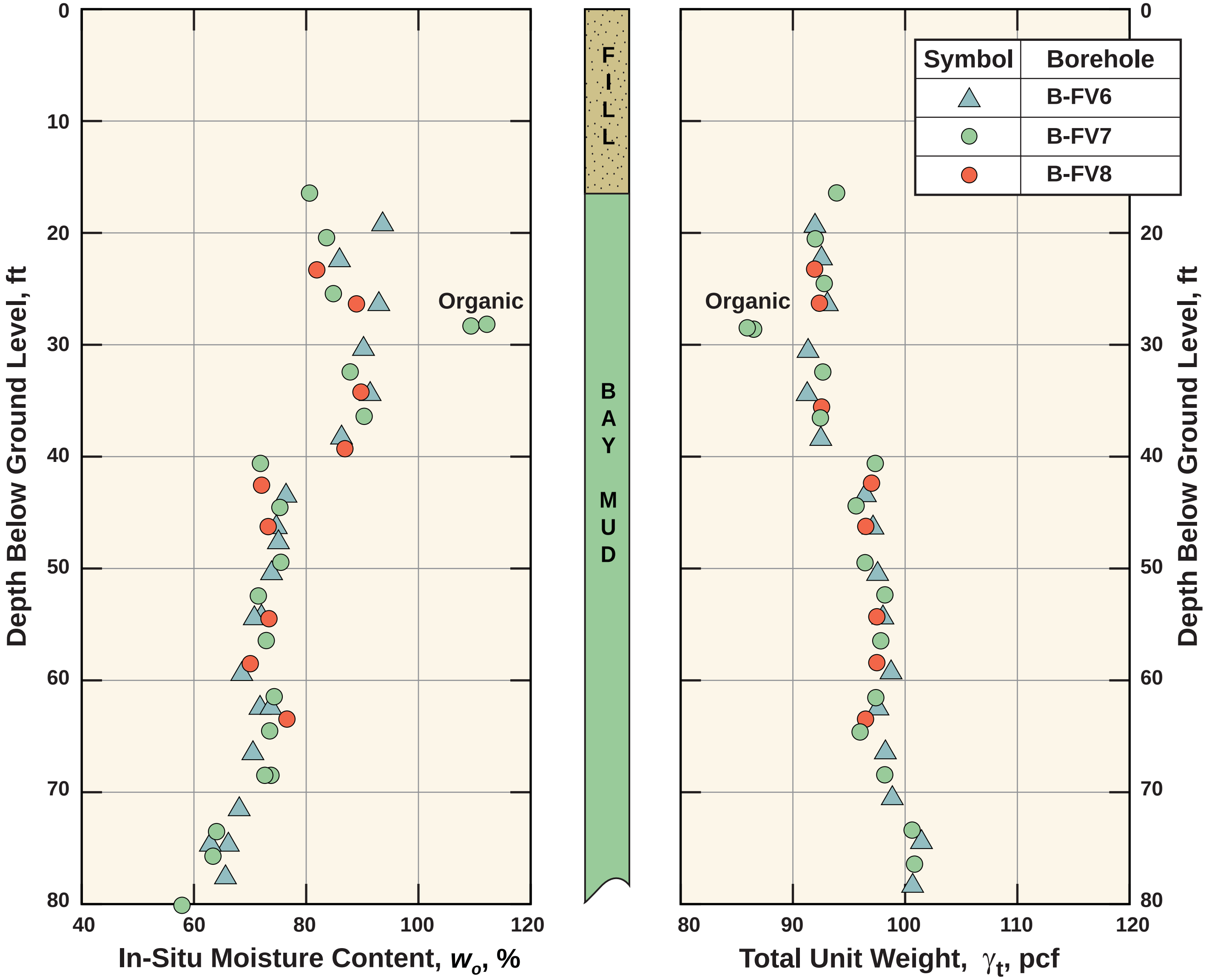


***FIG_20: Index Properties of Bay Mud Islais Creek Area Marshlands (Contract C Alignment)***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_20*

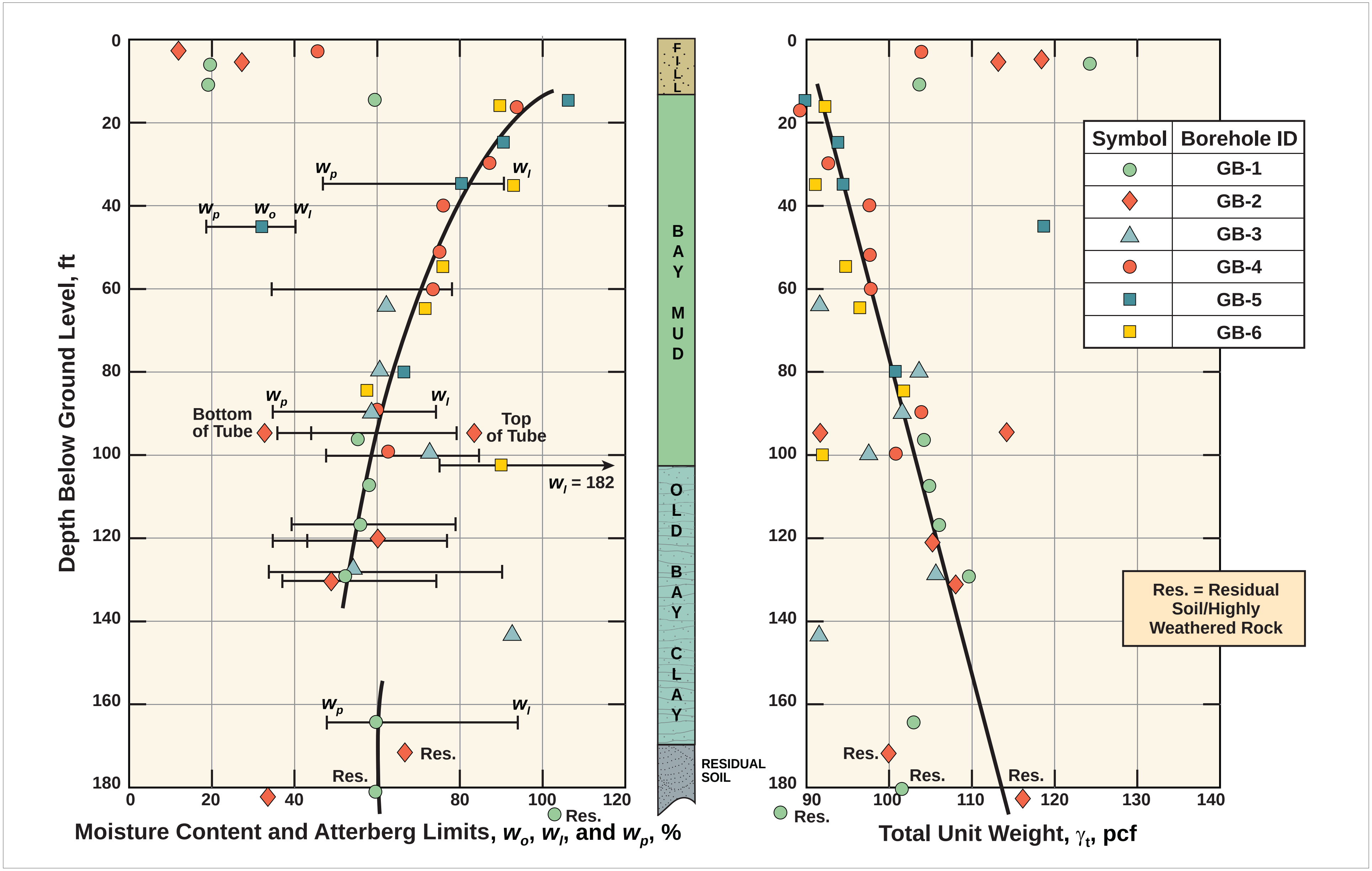


*FIG_21: Index Properties of Bay Mud Islais Creek Channel Area (Digesters Project)*

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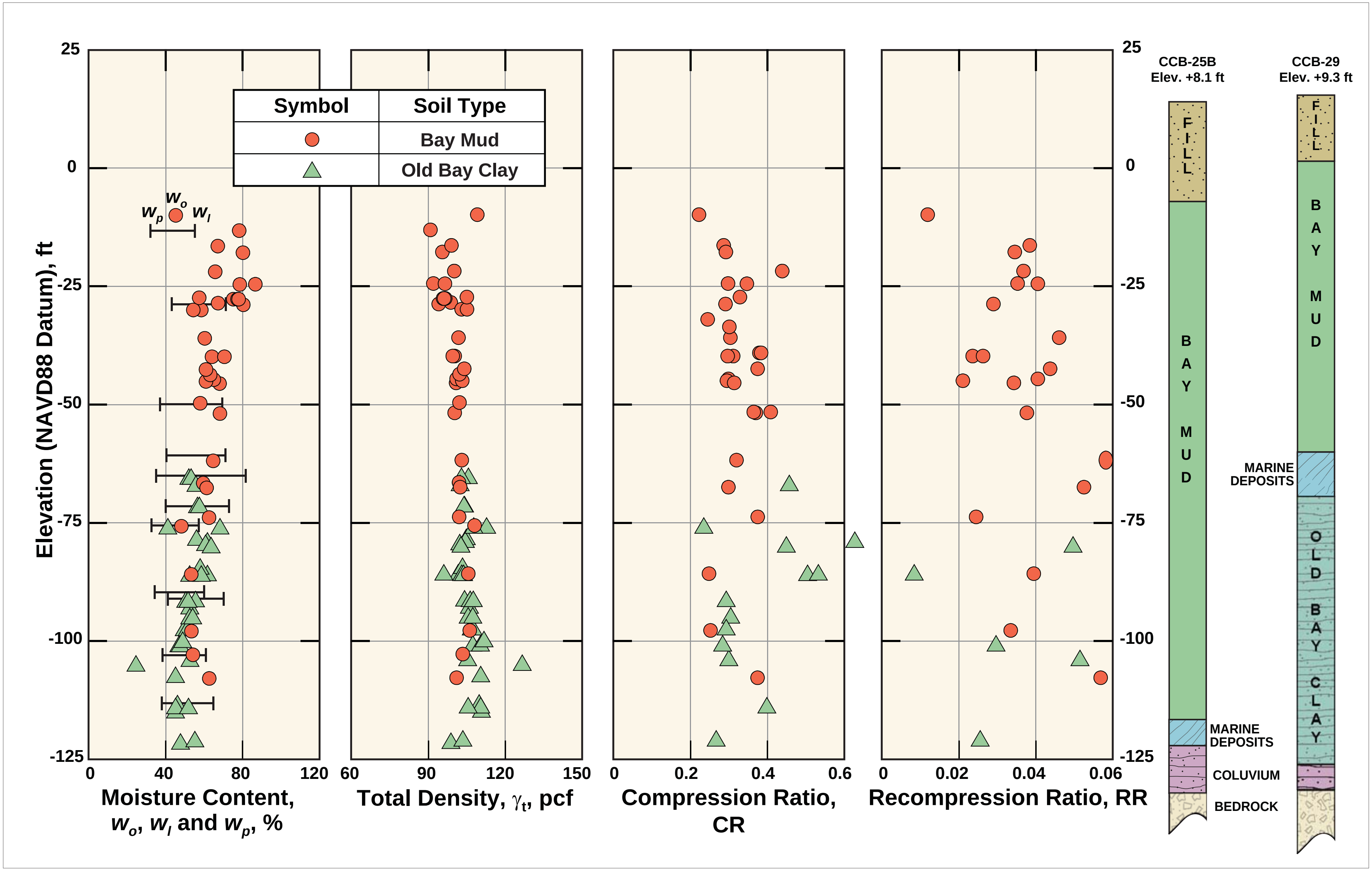
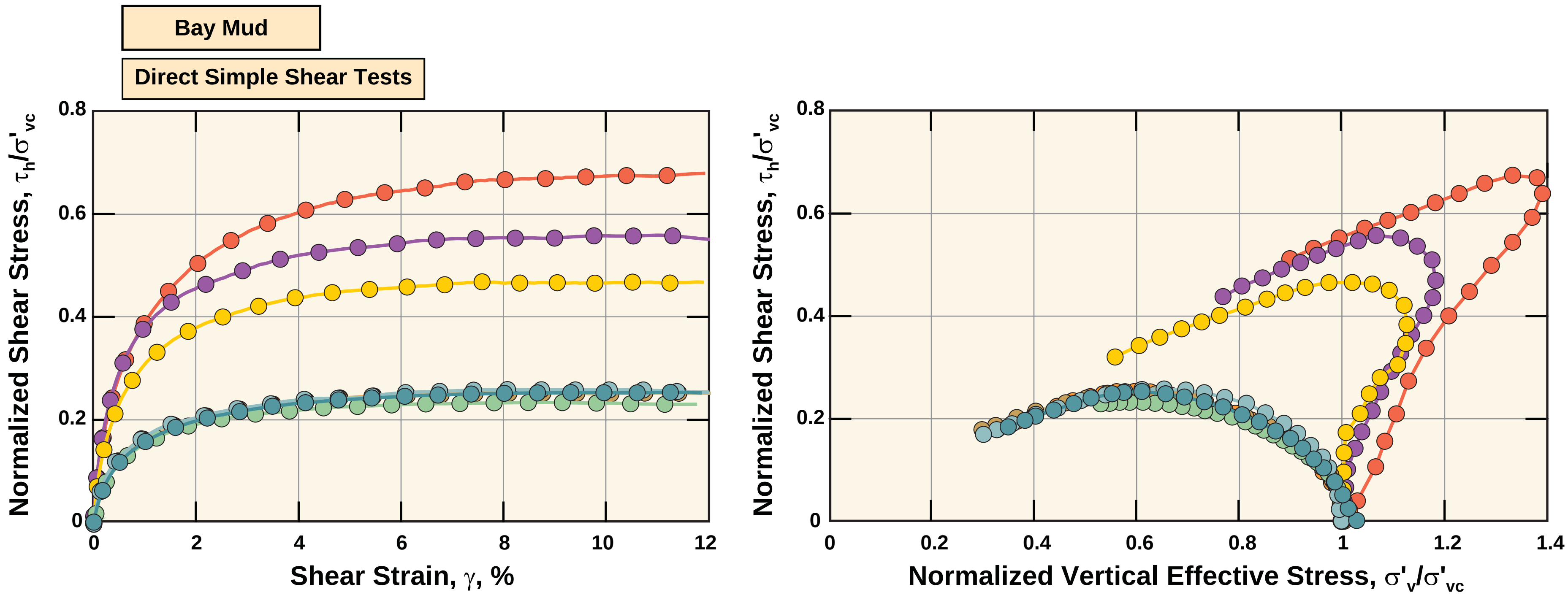



*FIG_22: Index Properties of Bay Mud DTX Segment Along Townsend Street (Mission Bay Area)*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_22*




| Symbol | OCR | $\sigma'_{vc}$ (psf) | $\tau_{h,max}/\sigma'_{vc}$ | $\gamma$ (%) |
|---|---|---|---|---|
| | 1.0 | 8,169 | 0.25 | 11.4 |
| | | 2,716 | 0.26 | 8.4 |
| | | 5,016 | 0.23 | 8.7 |
| | | 5,411 | 0.25 | 8.3 |
| | | 5,378 | 0.25 | 8.6 |
| | 2.0 | 2,728 | 0.47 | 7.5 |
| | 2.4 | 1,780 | 0.56 | 11.0 |
| | 3.0 | 1,894 | 0.68 | 11.9 |

*FIG_23: Normalized Undrained Strength Behavior of Bay Mud DSS Tests*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_23*

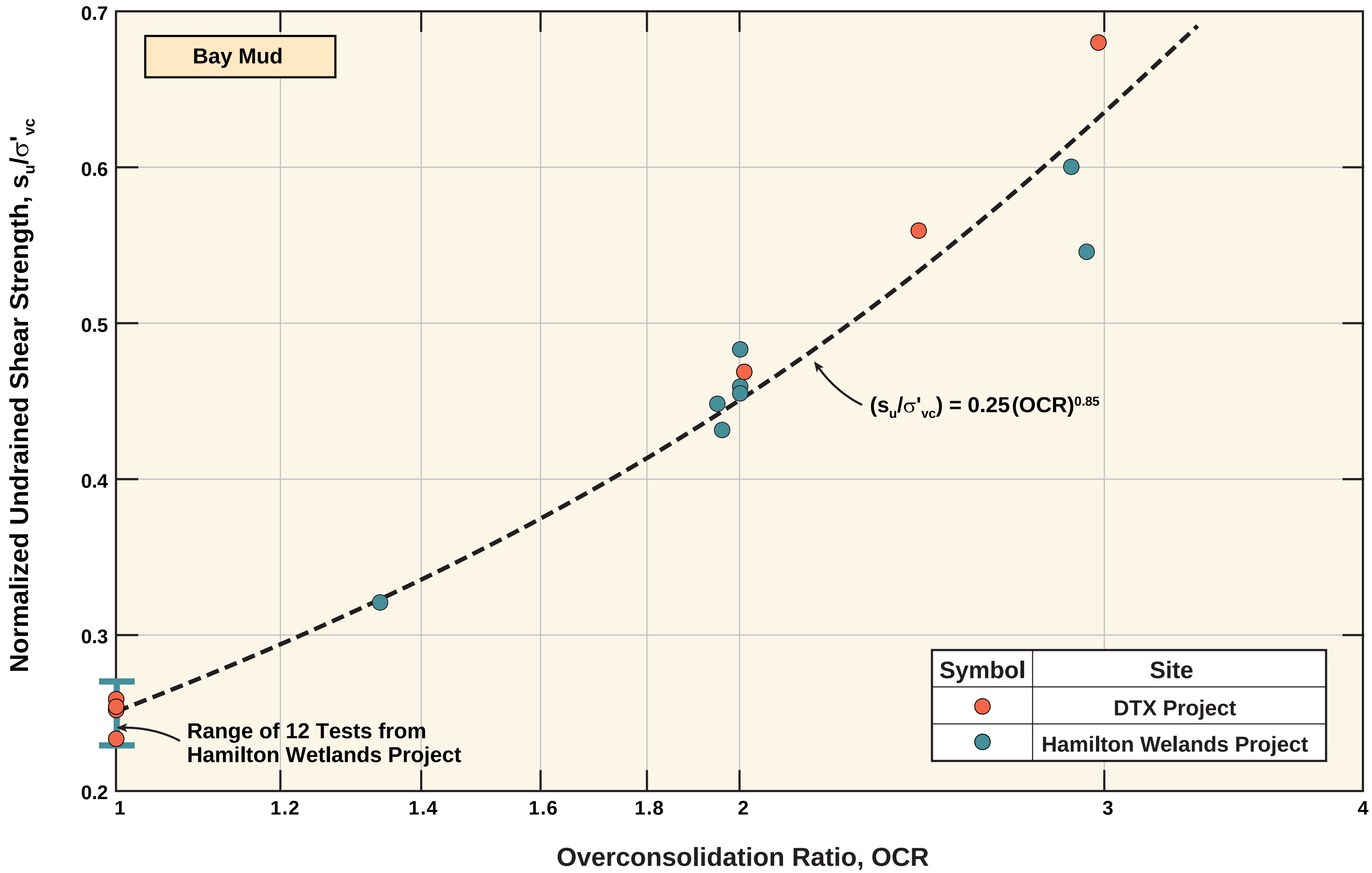



***FIG_24: Variation of Normalized Strength Ratios with Overconsolidation Ratio***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_24*

## ① Undrained Shear Strength

$$s_u = \sigma'_v S(OCR)^m, \quad \text{where } S = (s_u/\sigma'_v)_{NC}$$

## ② Overconsolidation Ratio, OCR

$$OCR = \frac{\sigma'_p}{\sigma'_{vo}} = \left[\frac{s_u/\sigma'_{vo}}{S}\right]^{1/m}$$

## ③ Maximum Past Pressure, $\sigma'_p$, from Field Vane Shear Tests

$$\sigma'_p = \sigma'_{vo}\left[\mu\frac{(s_u)_{FV}/\sigma'_{vo}}{S_{DSS}}\right]^{1/m},$$

where, $S_{DSS} = (s_u/\sigma'_{vc})_{NC}$ from Direct Simple Shear Tests

$\mu$ = Field Shear Vane Correction Factor

*FIG_25: SHANSEP Principles of Normalized Undrained Strength Behavior*

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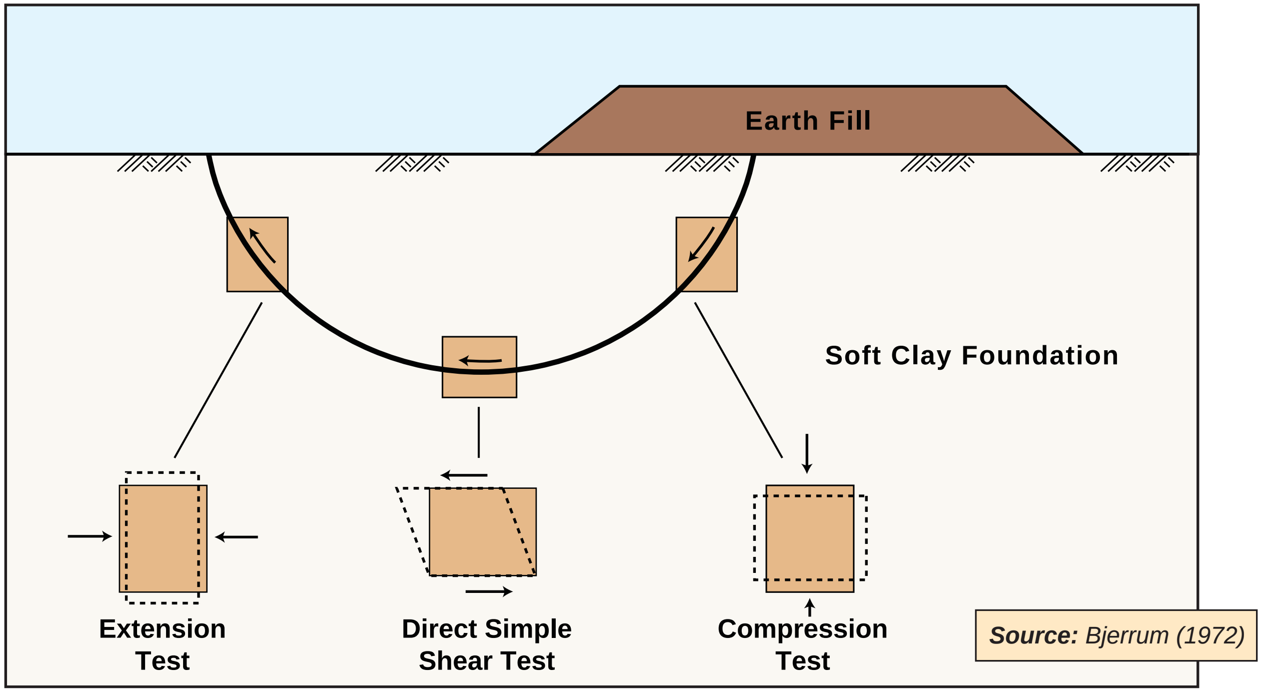


| Type of Soil | Index Properties % | | | | Triaxial Tests $\tau/p_o$ | | Simple Shear Tests | Vane Tests $S_u/p_o$ | |
|---|---|---|---|---|---|---|---|---|---|
| | w | $w_L$ | $w_p$ | $I_p$ | Compres. | Extension | $\tau_h/p_o$ | Observed | Corrected for Rate |
| Bangkok Clay | 140 | 150 | 65 | 85 | 0.70 | 0.40 | 0.41 | 0.59 | 0.47 |
| Matagami Clay | 90 | 85 | 38 | 47 | 0.61 | 0.45 | 0.39 | 0.46 | 0.40 |
| Drammen Plastic Clay | 52 | 61 | 32 | 29 | 0.40 | 0.15 | 0.30 | 0.36 | 0.30 |
| Vaterland Clay | 35 | 42 | 26 | 16 | 0.32 | 0.09 | 0.26 | 0.22 | 0.20 |
| Studenterlunden | 31 | 43 | 25 | 18 | 0.31 | 0.10 | 0.19 | 0.18 | 0.16 |
| Drammen Lean Clay | 30 | 33 | 22 | 11 | 0.34 | 0.09 | 0.22 | 0.24 | 0.21 |

***FIG_26: Anisotropic Behavior of Soft Clays***

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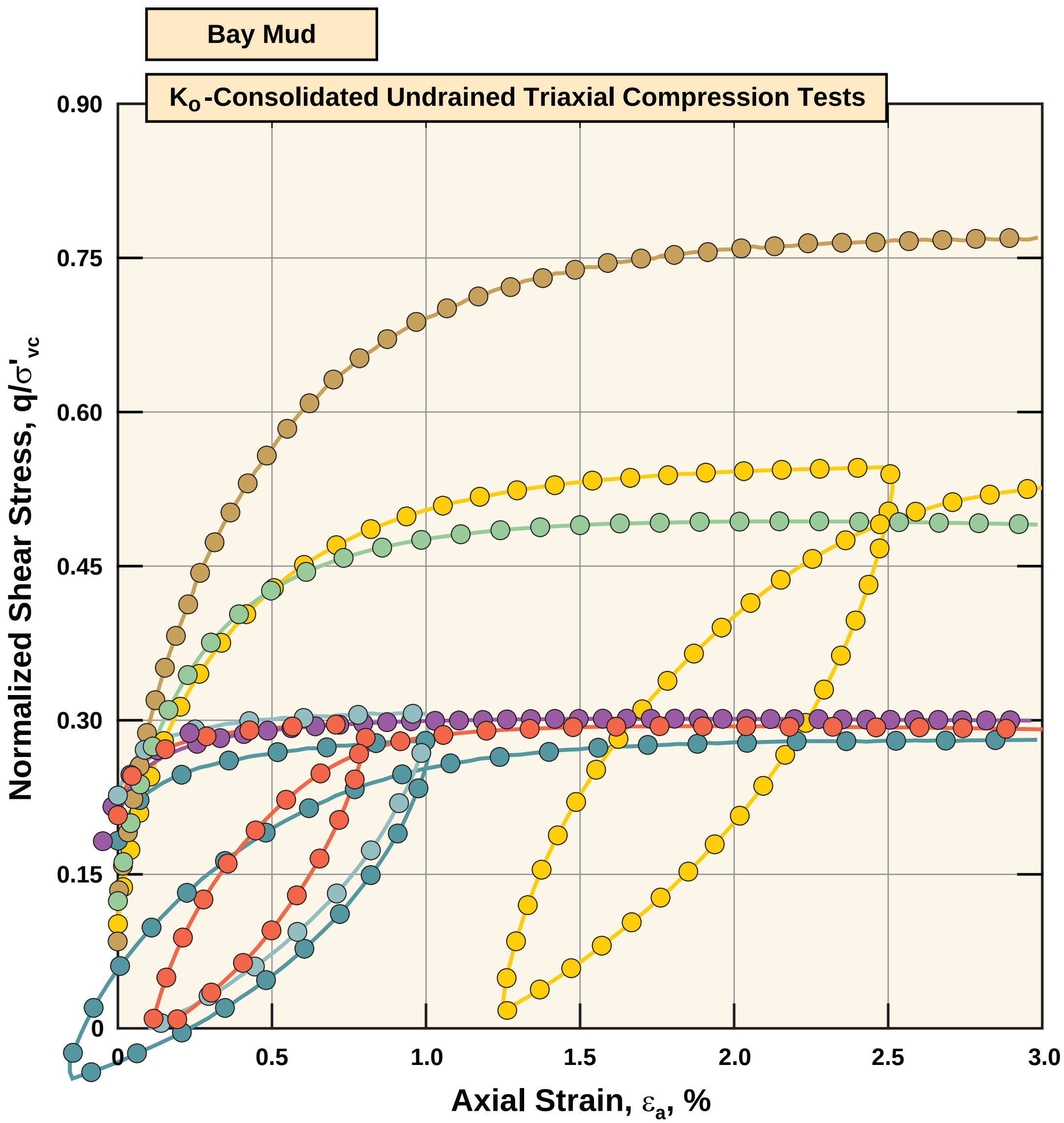


| Symbol | OCR | $\sigma'_{vc}$ (psf) | $s_u/\sigma'_{vc}$ | $\varepsilon_a$ (%) |
|---|---|---|---|---|
| red | 1.0 | 10,099 | 0.30 | 0.8 |
| light blue | | 4,055 | 0.31 | 1.0 |
| purple | | 4,239 | 0.30 | 1.6 |
| teal | | 12,188 | 0.28 | 3.2 |
| green | 1.8 | 4,026 | 0.49 | 2.2 |
| yellow | | 2,527 | 0.55 | 2.5 |
| tan | 2.8 | 2,048 | 0.77 | 3.6 |

*FIG_27: Normalized Strength-Strain Behavior from Undrained Ko-Triaxial Compression Tests*

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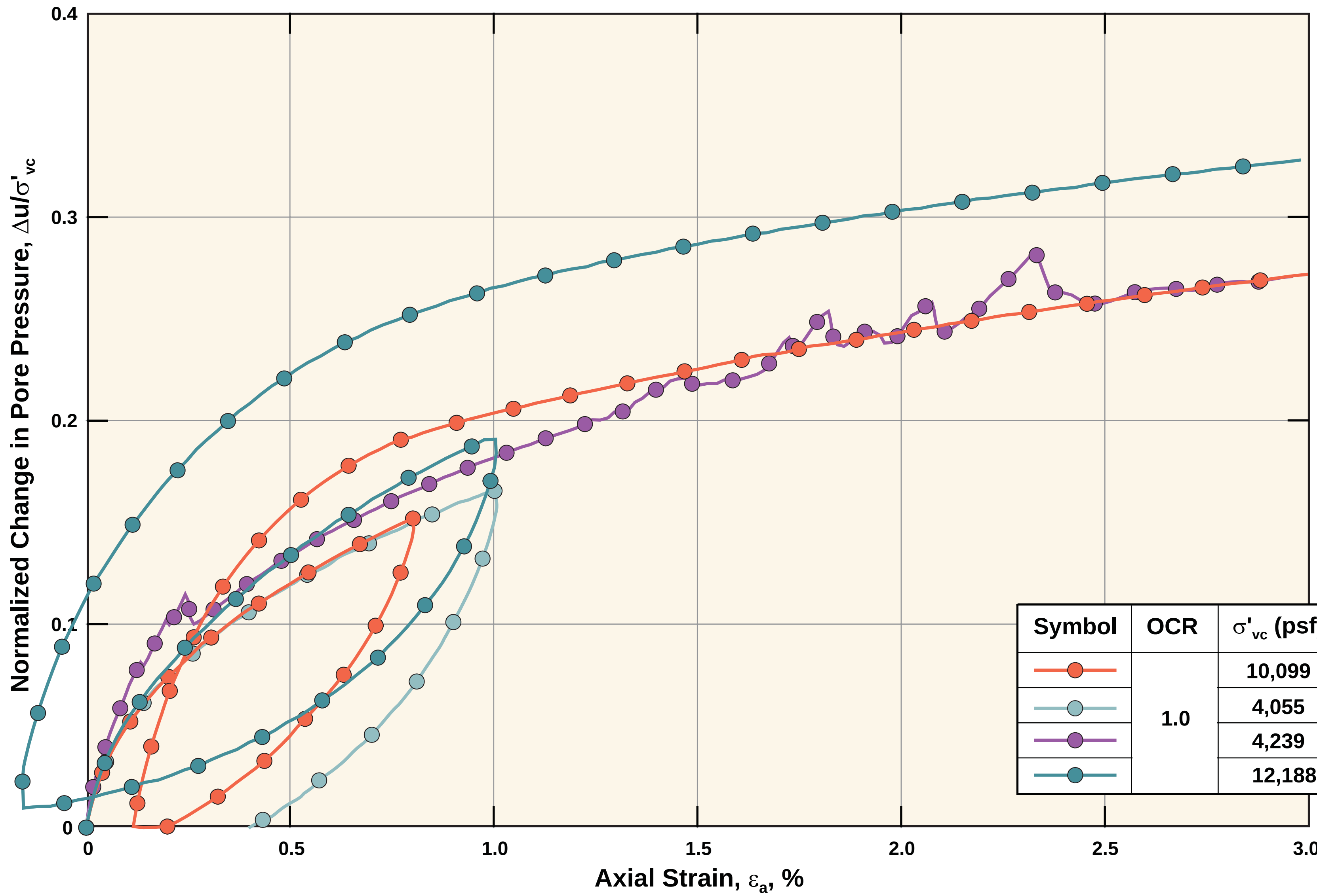


| Symbol | OCR | $\sigma'_{vc}$ (psf) | $s_u/\sigma'_{vc}$ |
|---|---|---|---|
| orange | 1.0 | 10,099 | 0.30 |
| light teal | | 4,055 | 0.31 |
| purple | | 4,239 | 0.30 |
| dark teal | | 12,188 | 0.28 |

***FIG_28: Pore Pressure Behavior from Ko-Triaxial Compression Tests***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_28.ai*

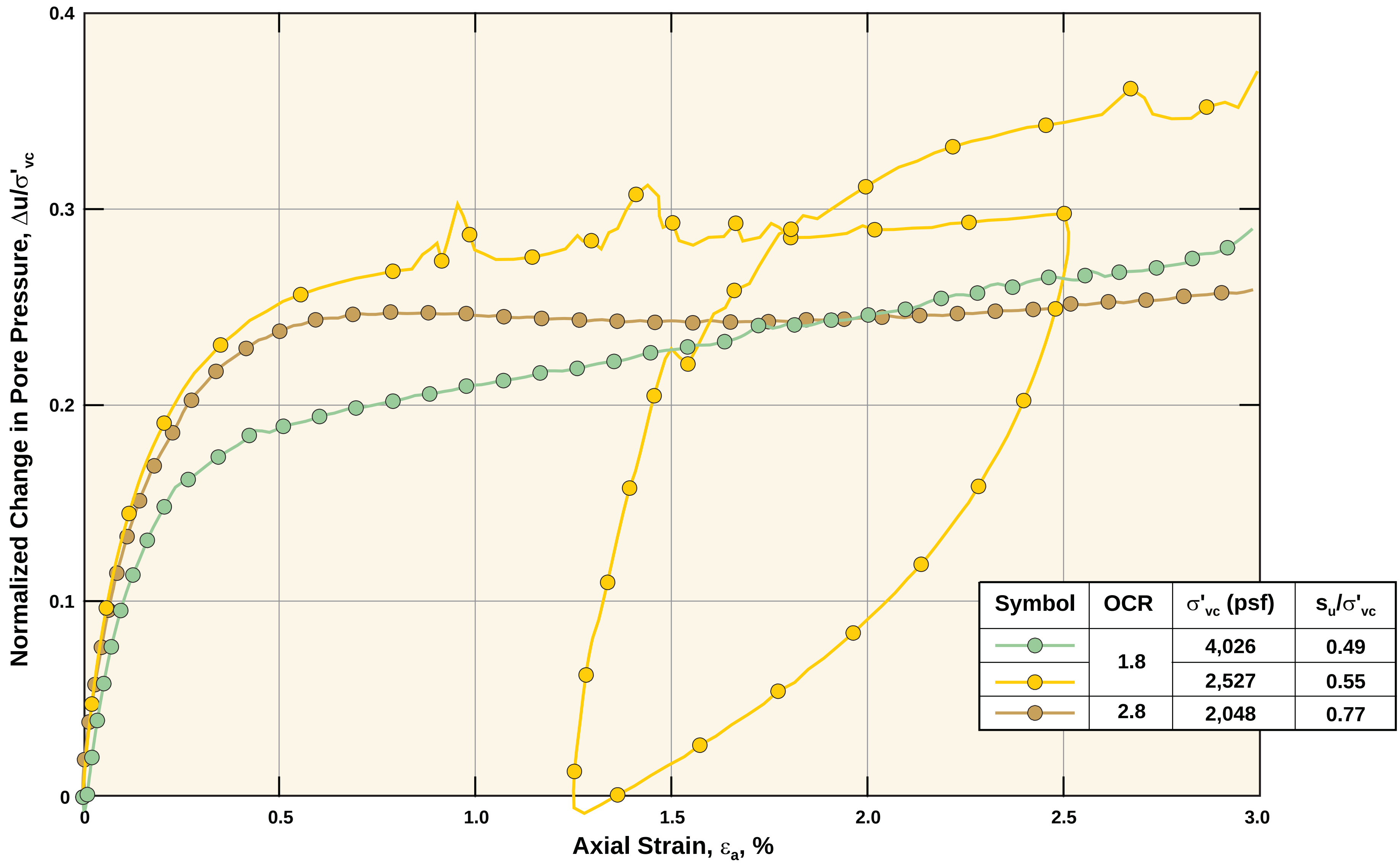


***FIG_28A: Pore Pressure Behavior from Ko-Triaxial Compression Tests***

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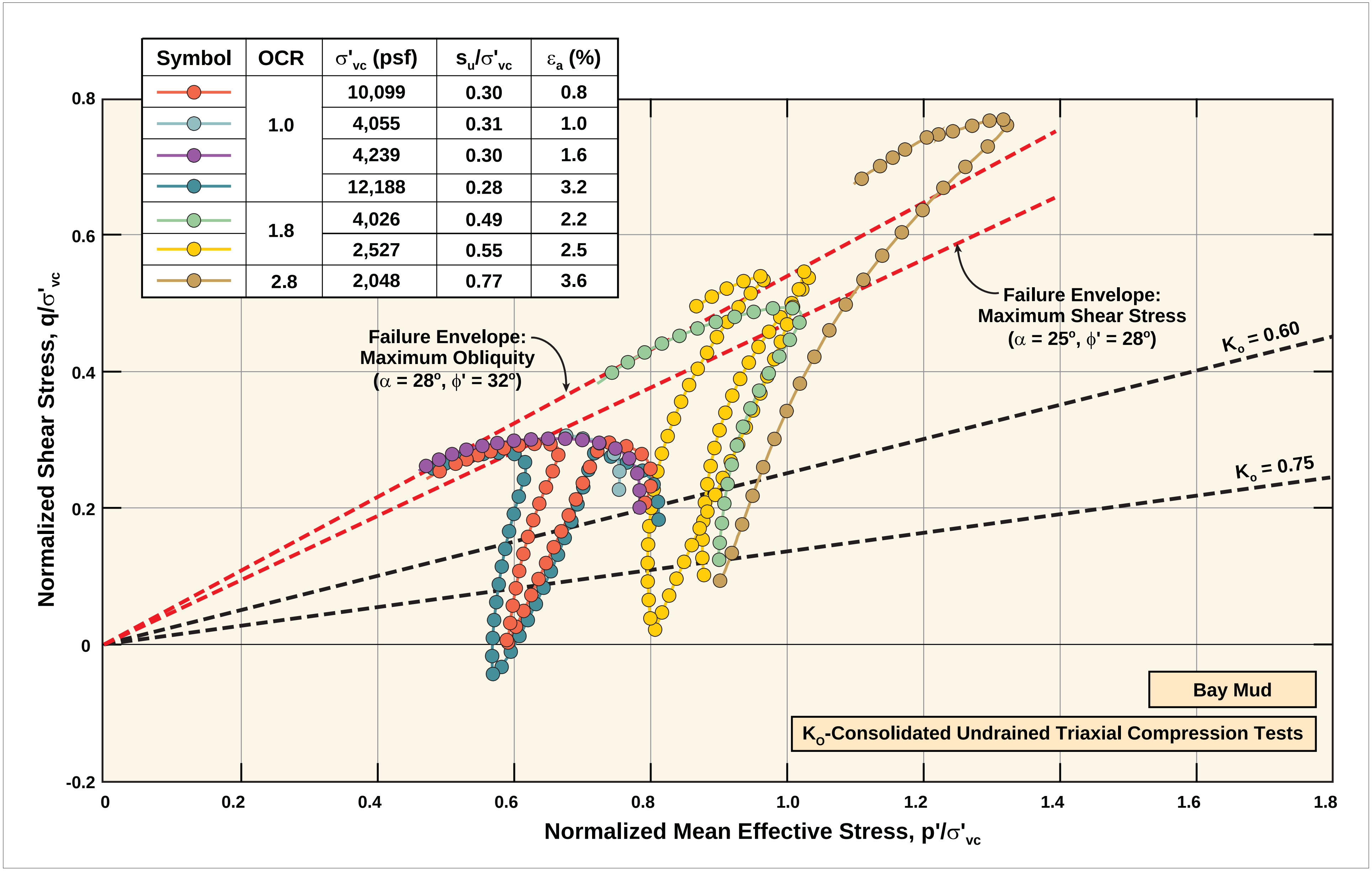


***FIG_29: Effective Stress Paths from Ko-Triaxial Compression Tests***

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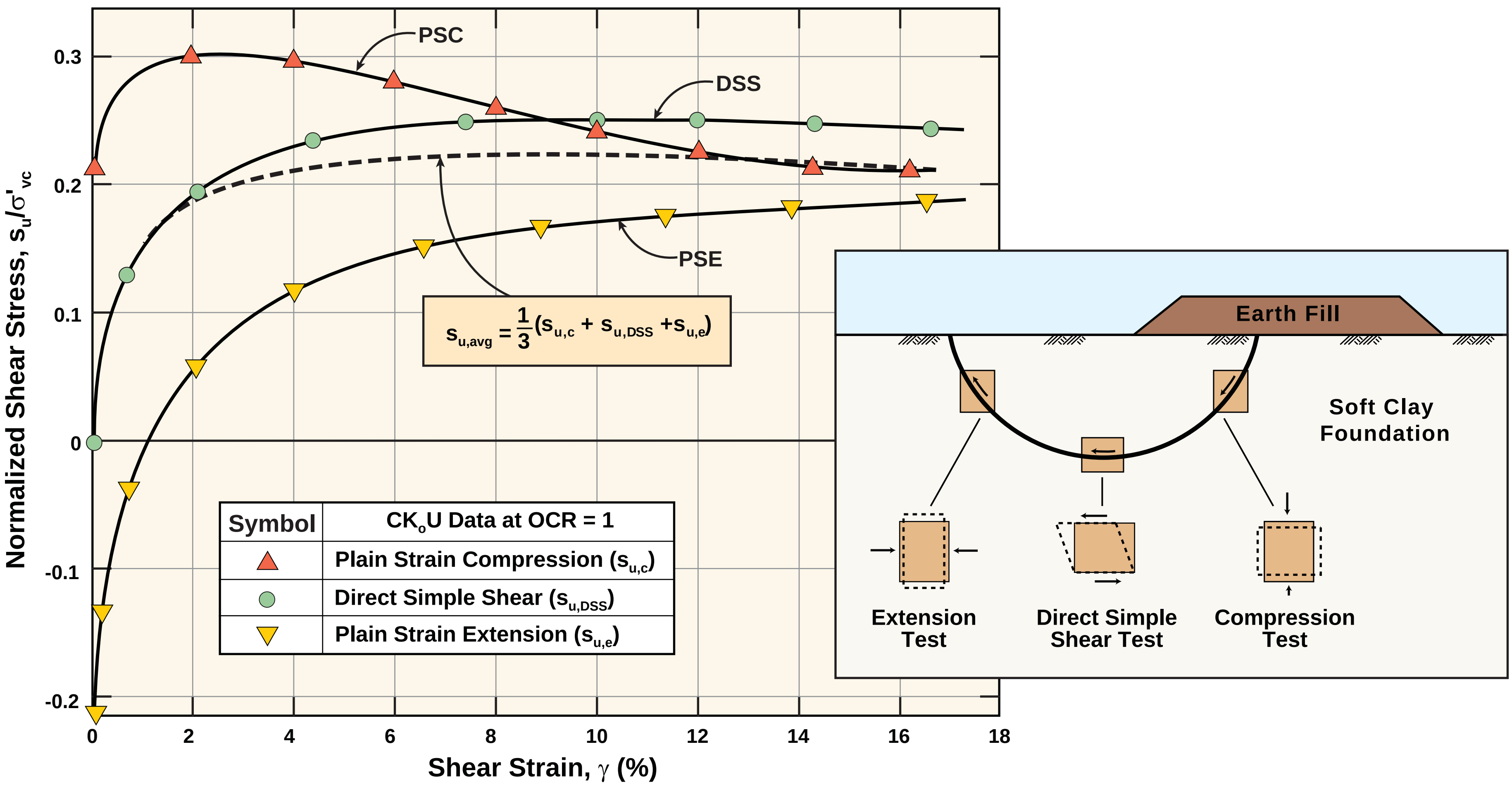


*FIG_30: Anisotropy and Strain Compatibility Considerations*

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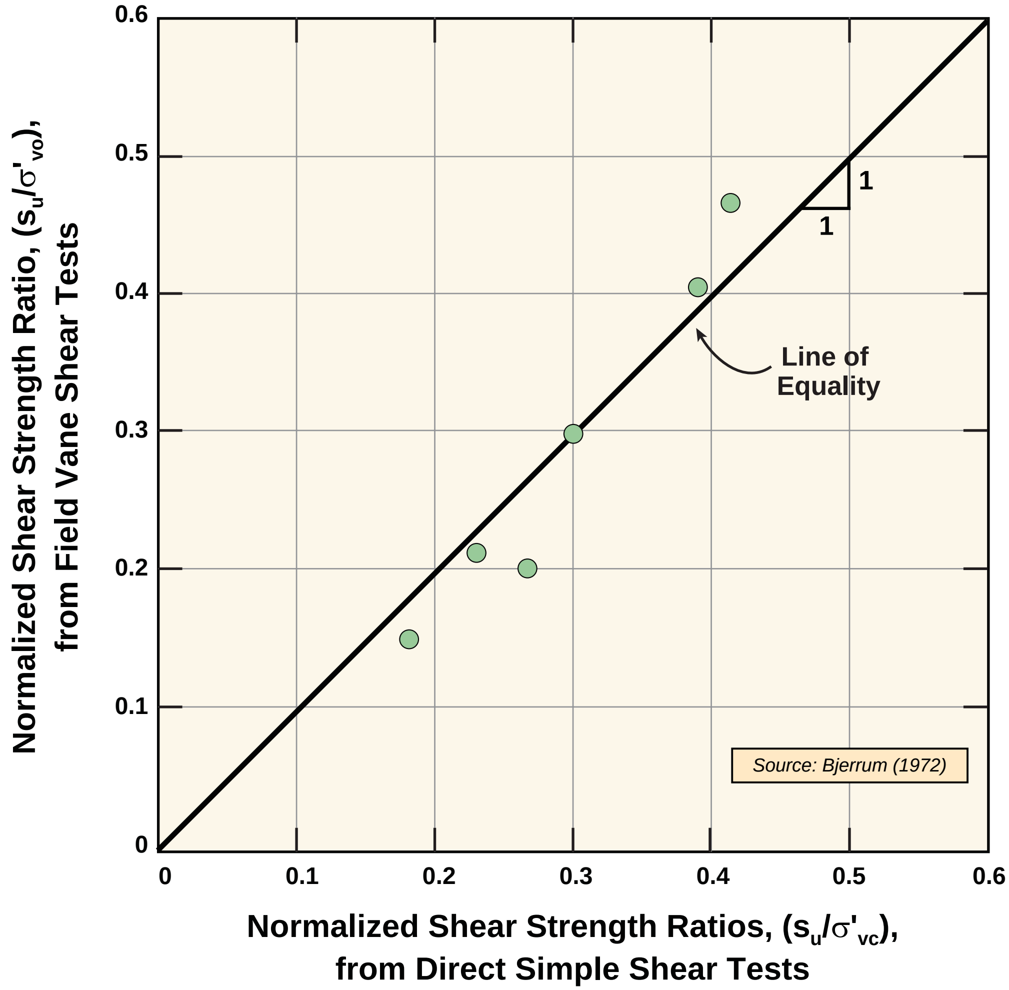


***FIG_31: Correlation of Normalized Strength Ratios from DSS and Vane Shear Tests***

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## ① Undrained Shear Strength

$$s_u = \sigma'_v S(OCR)^m, \quad \text{where } S = (s_u/\sigma'_v)_{NC}$$

## ② Overconsolidation Ratio, OCR

$$OCR = \frac{\sigma'_p}{\sigma'_{vo}} = \left[\frac{s_u/\sigma'_{vo}}{S}\right]^{1/m}$$

## ③ Maximum Past Pressure, $\sigma'_p$, from Field Vane Shear Tests

$$\sigma'_p = \sigma'_{vo}\left[\mu\frac{(s_u)_{FV}/\sigma'_{vo}}{S_{DSS}}\right]^{1/m},$$

where, $S_{DSS} = (s_u/\sigma'_{vc})_{NC}$ from Direct Simple Shear Tests

$\mu$ = Field Shear Vane Correction Factor

*FIG_32: SHANSEP Principles of Normalized Undrained Strength Behavior*

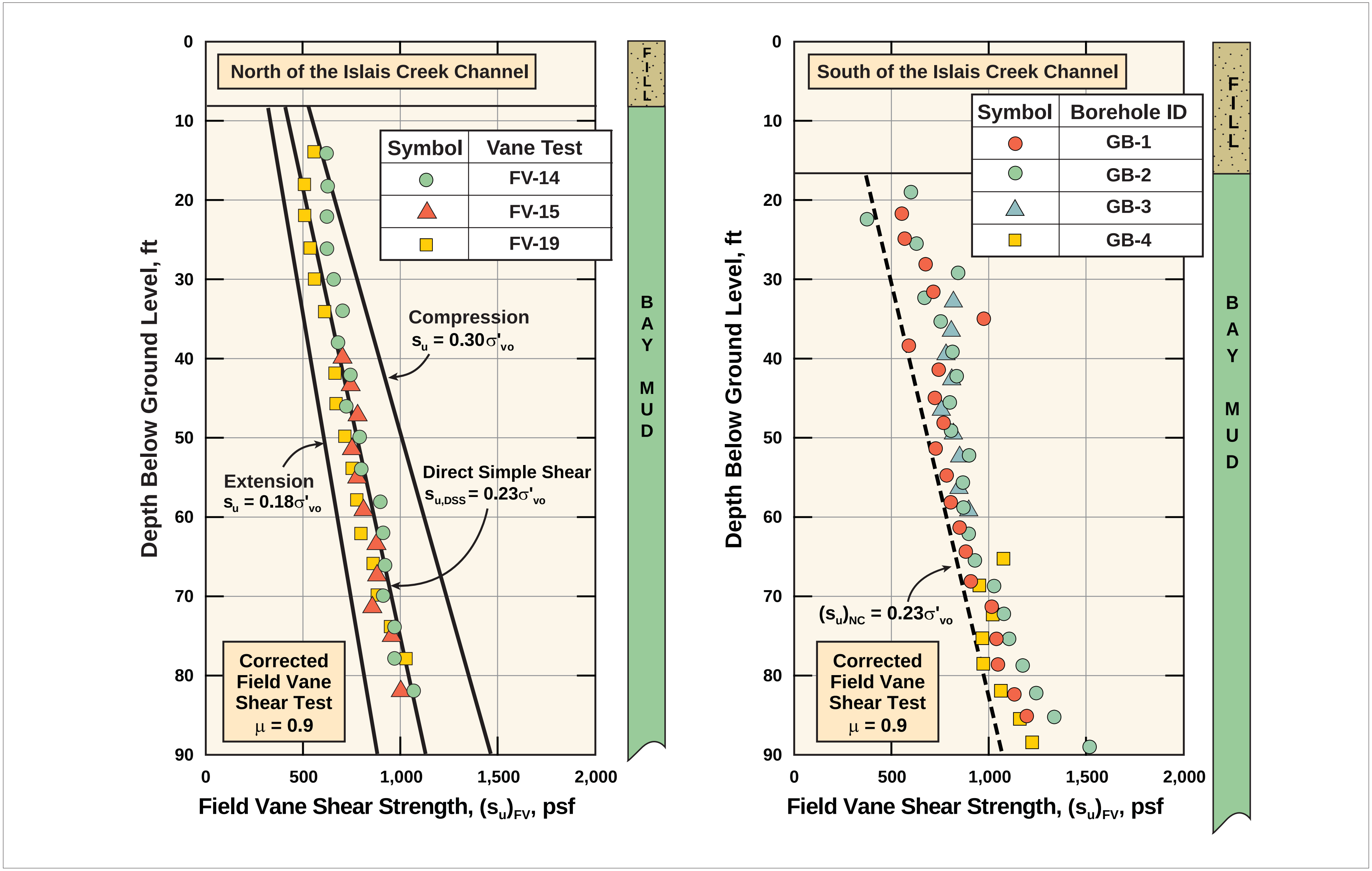


*FIG_33: Undrained Shear Strengths from Vane Shear Tests Islais Creek T/S Project (Contract D)*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)FIG_33*

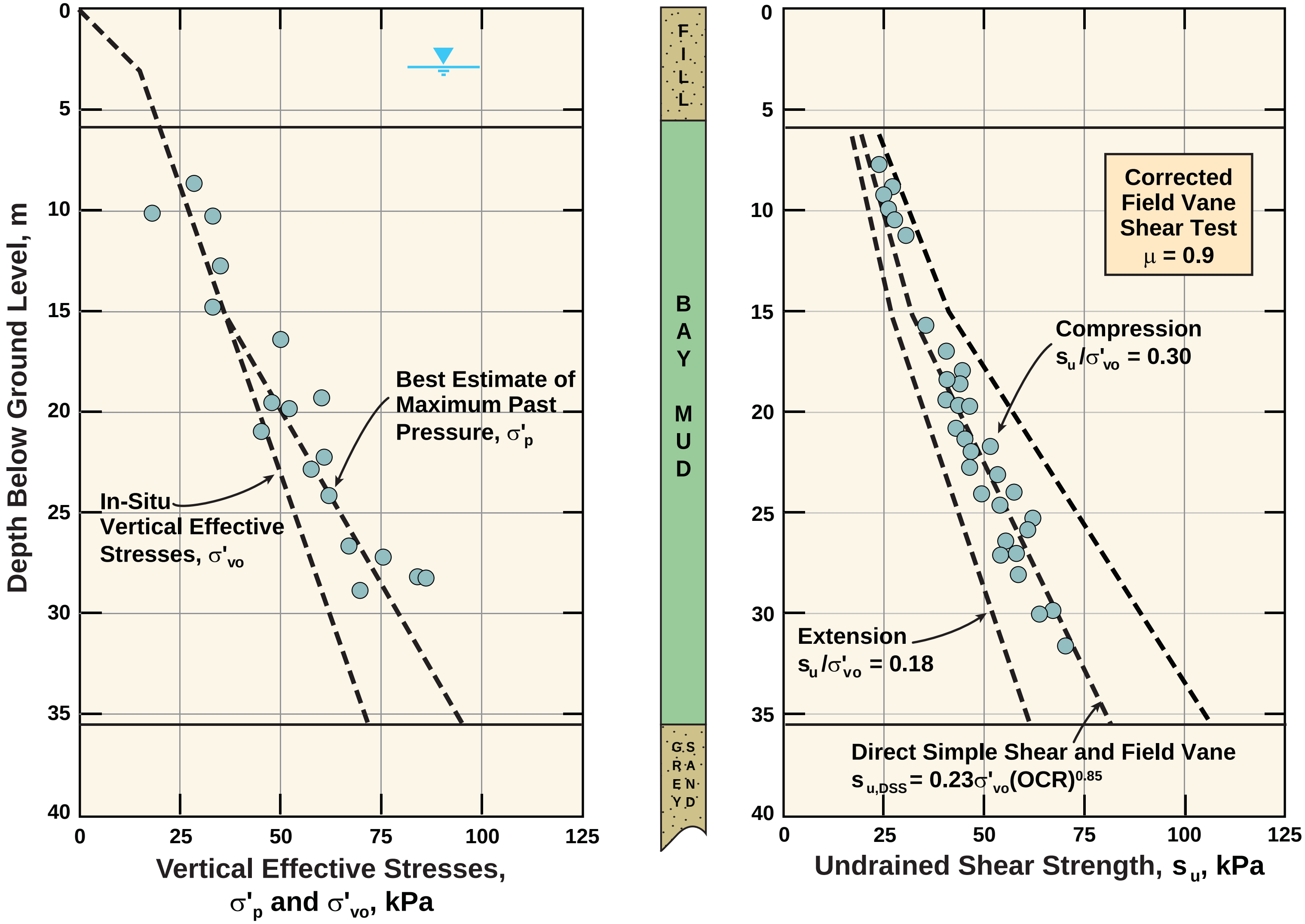


*FIG_34: Stress History and Vane Shear Strengths Muni Metro Turnback Site*

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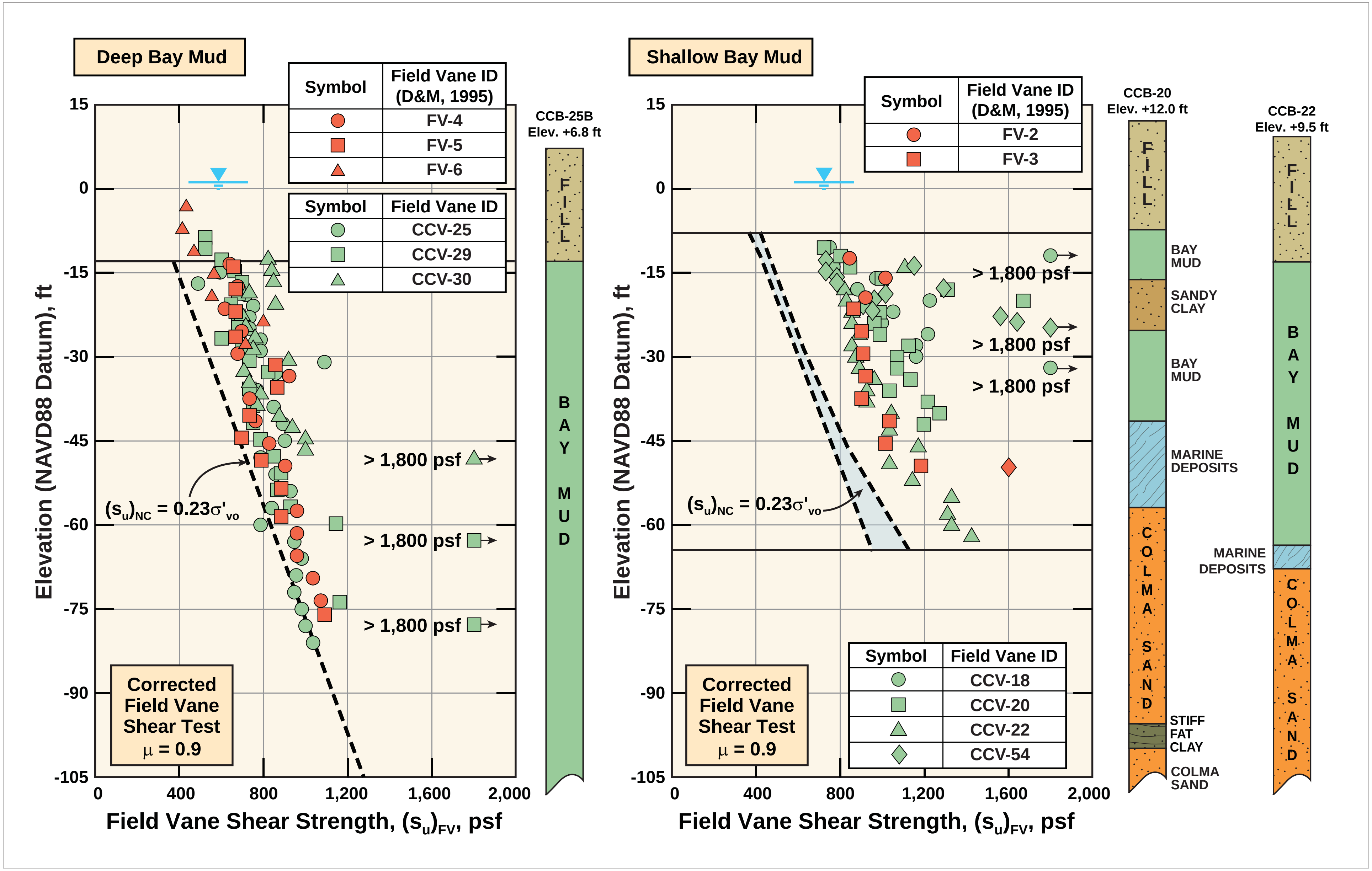


*FIG_35: Undrained Shear Strengths from Vane Shear Tests DTX Alignment Along Townsend Street*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_35*

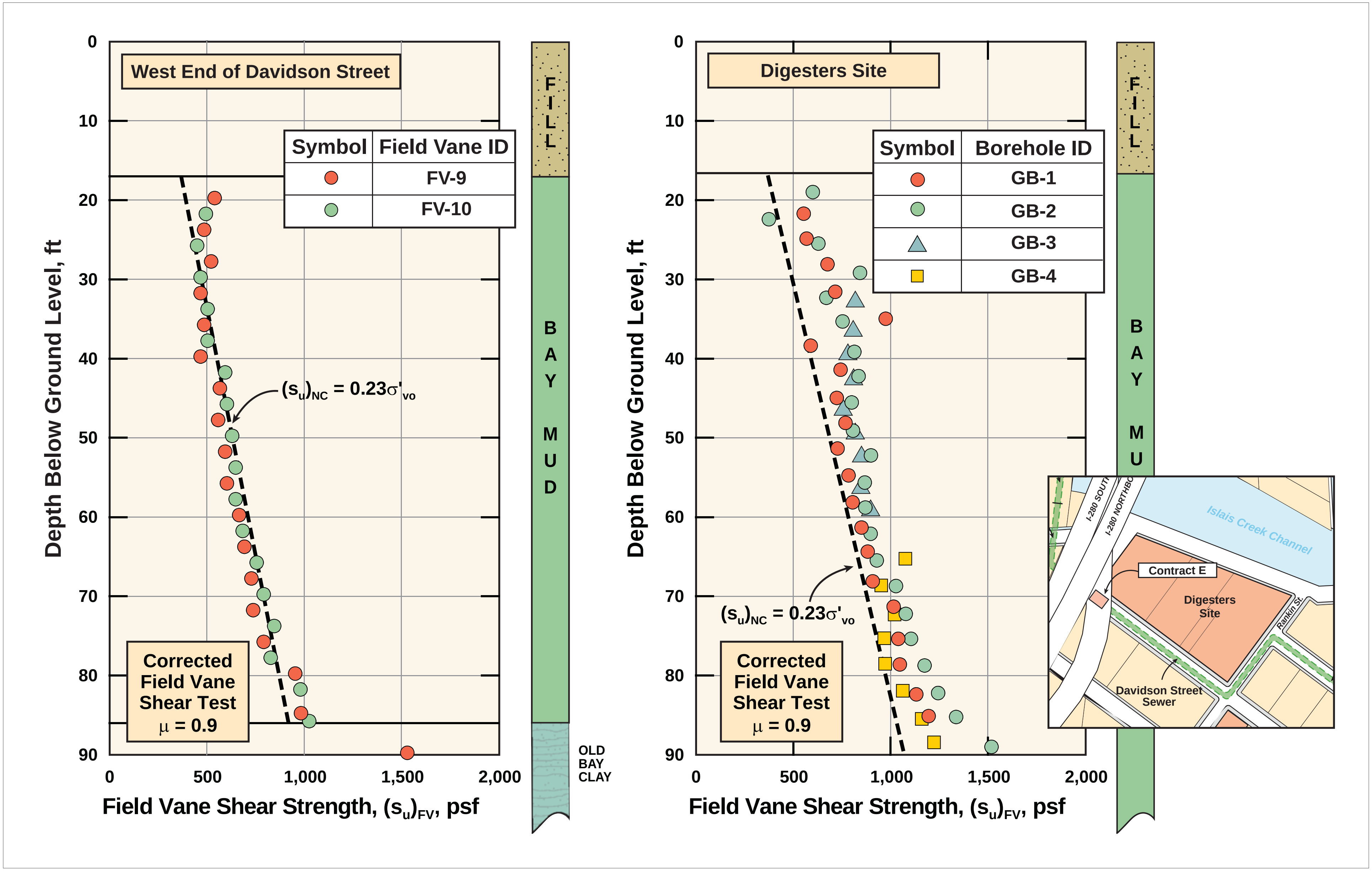


*FIG_36: Undrained Shear Strengths from Vane Shear Tests Islais Creek Area Along Davidson Street*

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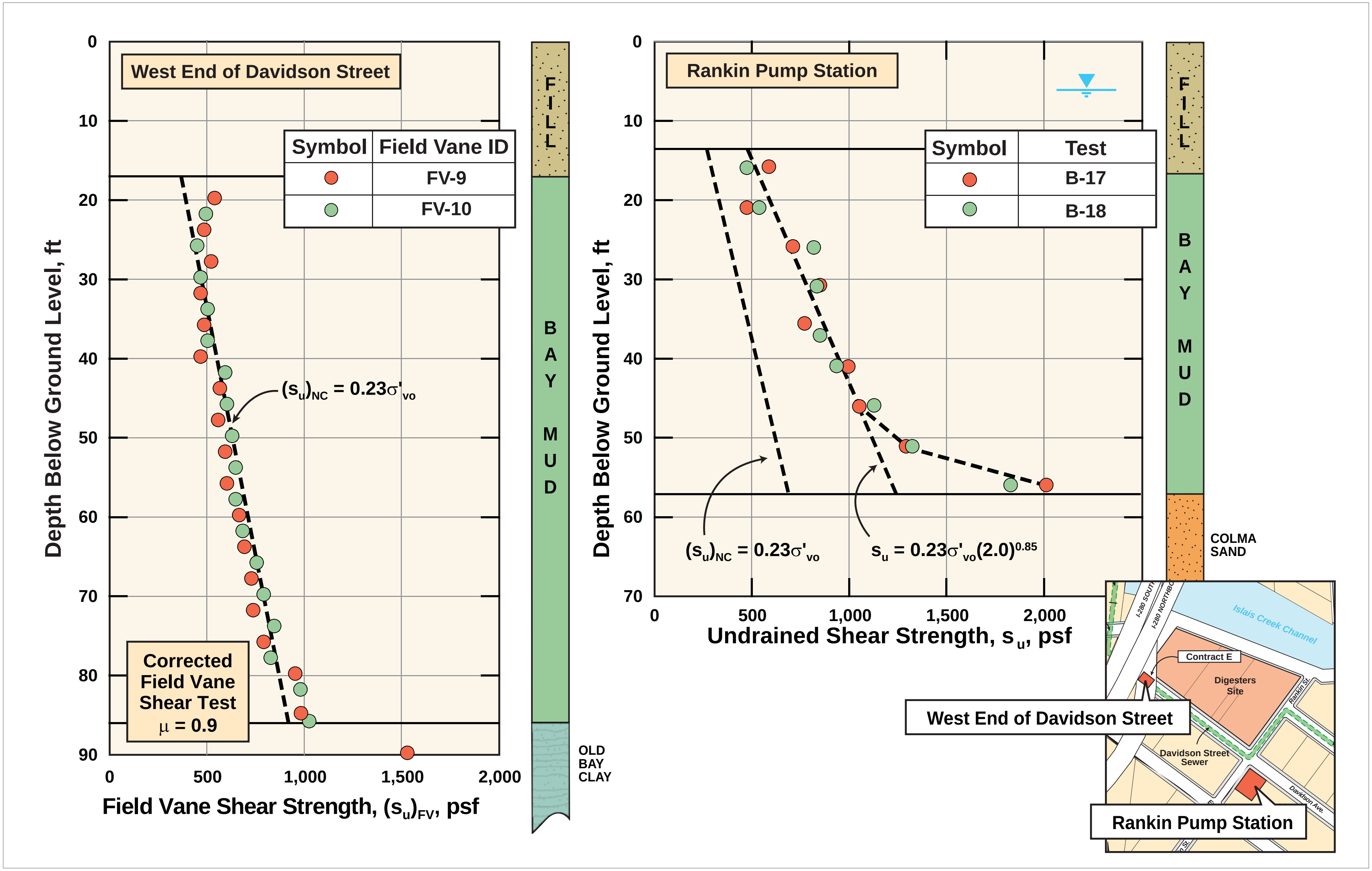


*FIG_36A: Undrained Shear Strengths from Vane Shear Tests Davidson Street: South of Islais Creek Channel*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_36*

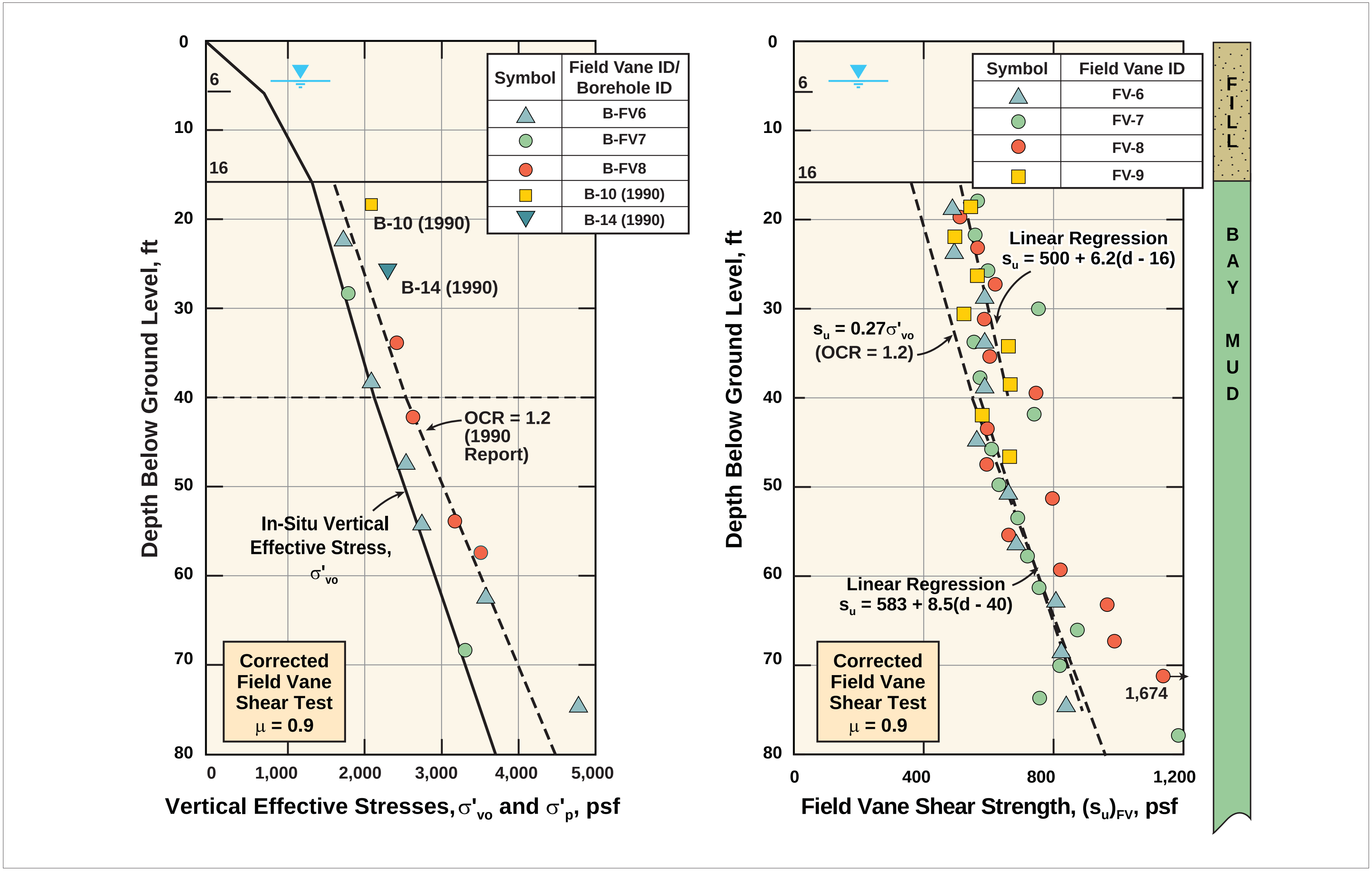


*FIG_37: Stress History and Undrained Strengths from Vane Shear and UU Tests Islais Creek (Contract C)*

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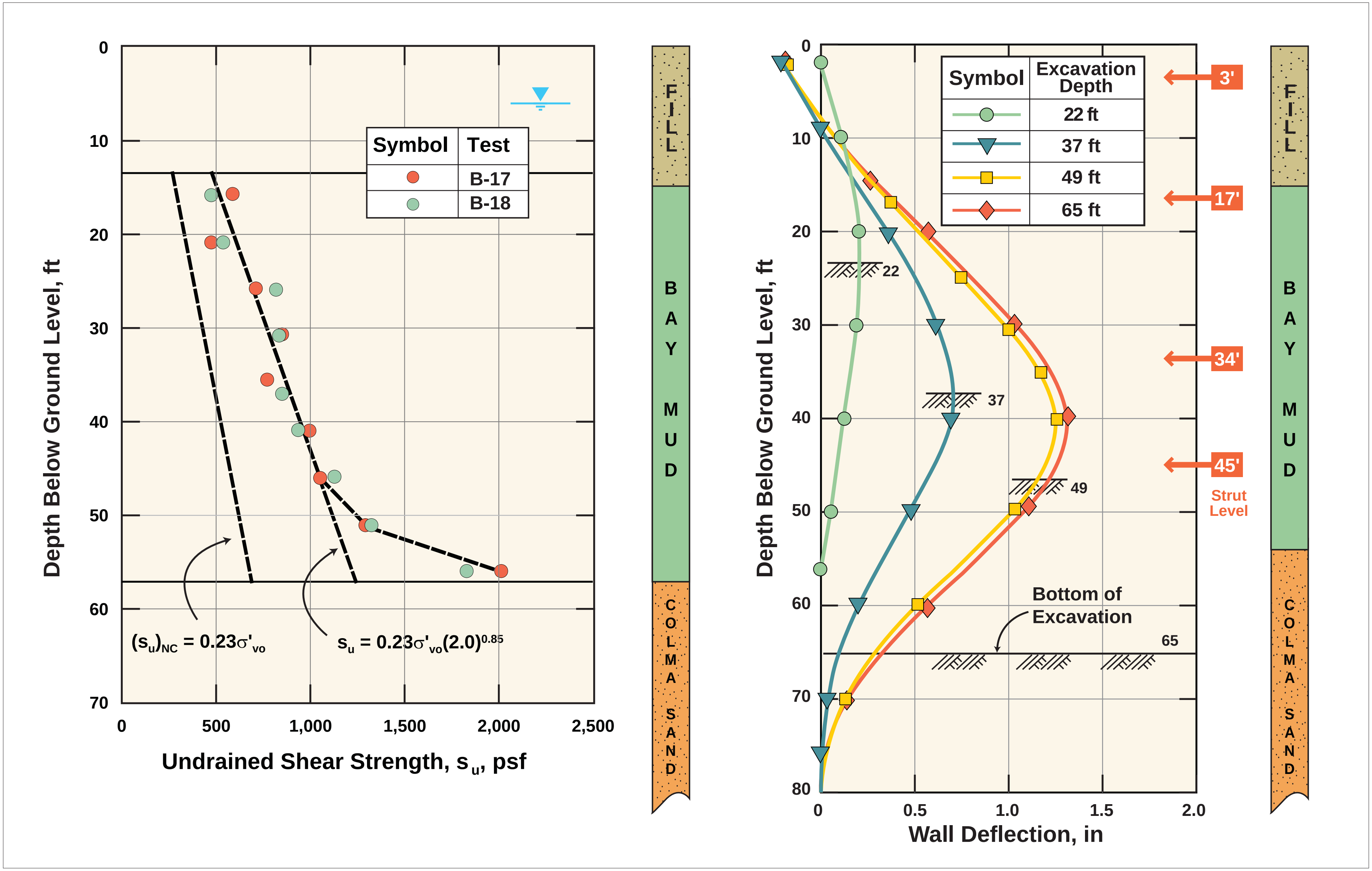


*FIG_37A: Undrained Shear Strengths from Vane Shear Tests Rankin Pump Station Site: Islais Creek Estuary*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_37A*

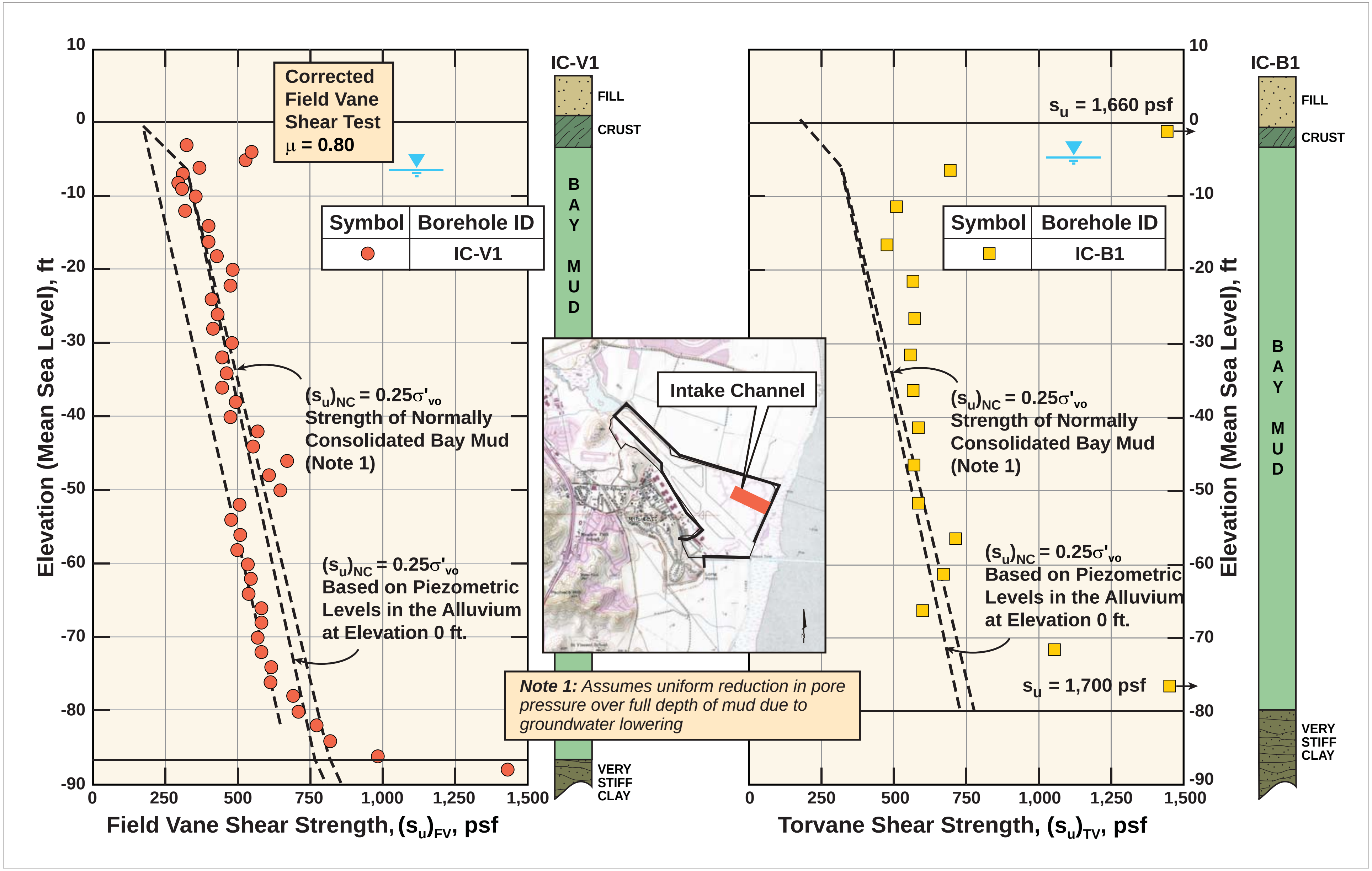


*FIG_38: Undrained Shear Strengths From Vane Shear and Torvane Tests Hamilton Field: Deep Mud Area*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_38*

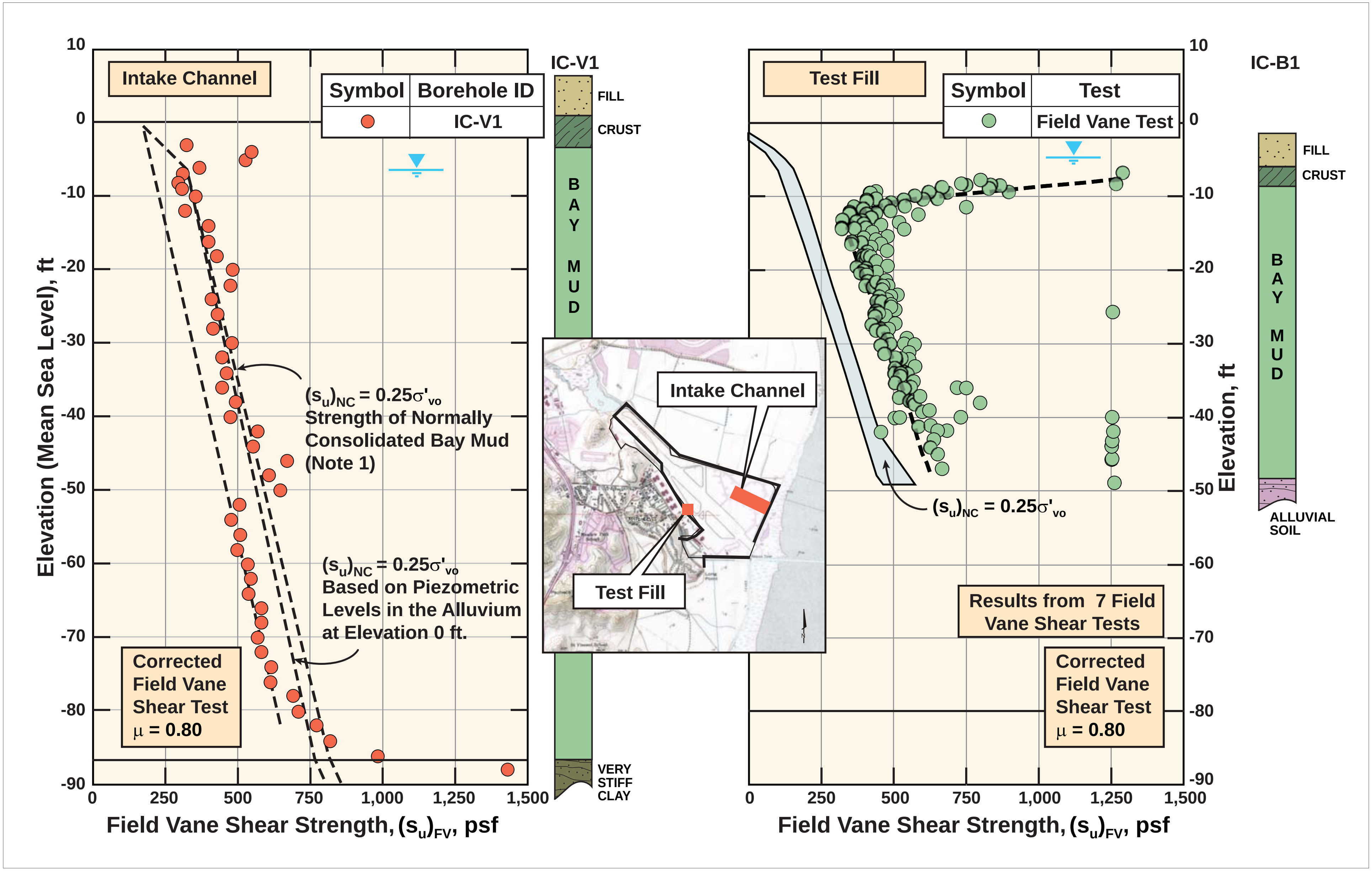


*FIG_38A: Undrained Shear Strengths From Field Vane Shear Tests Hamilton Field*

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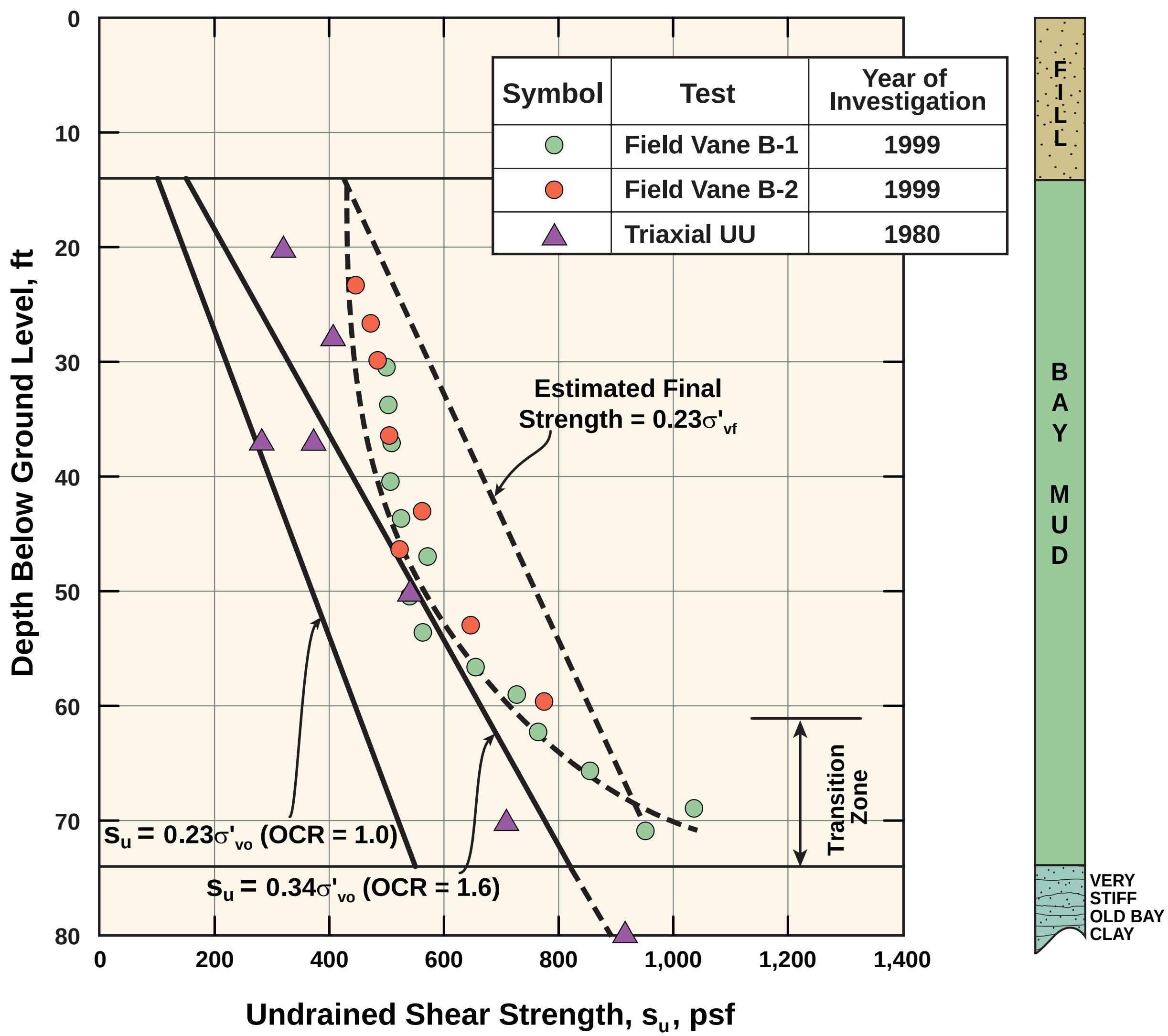


*FIG_39: Undrained Shear Strength of a Site Undergoing Consolidation Under "Recent" Reclamation Fill*

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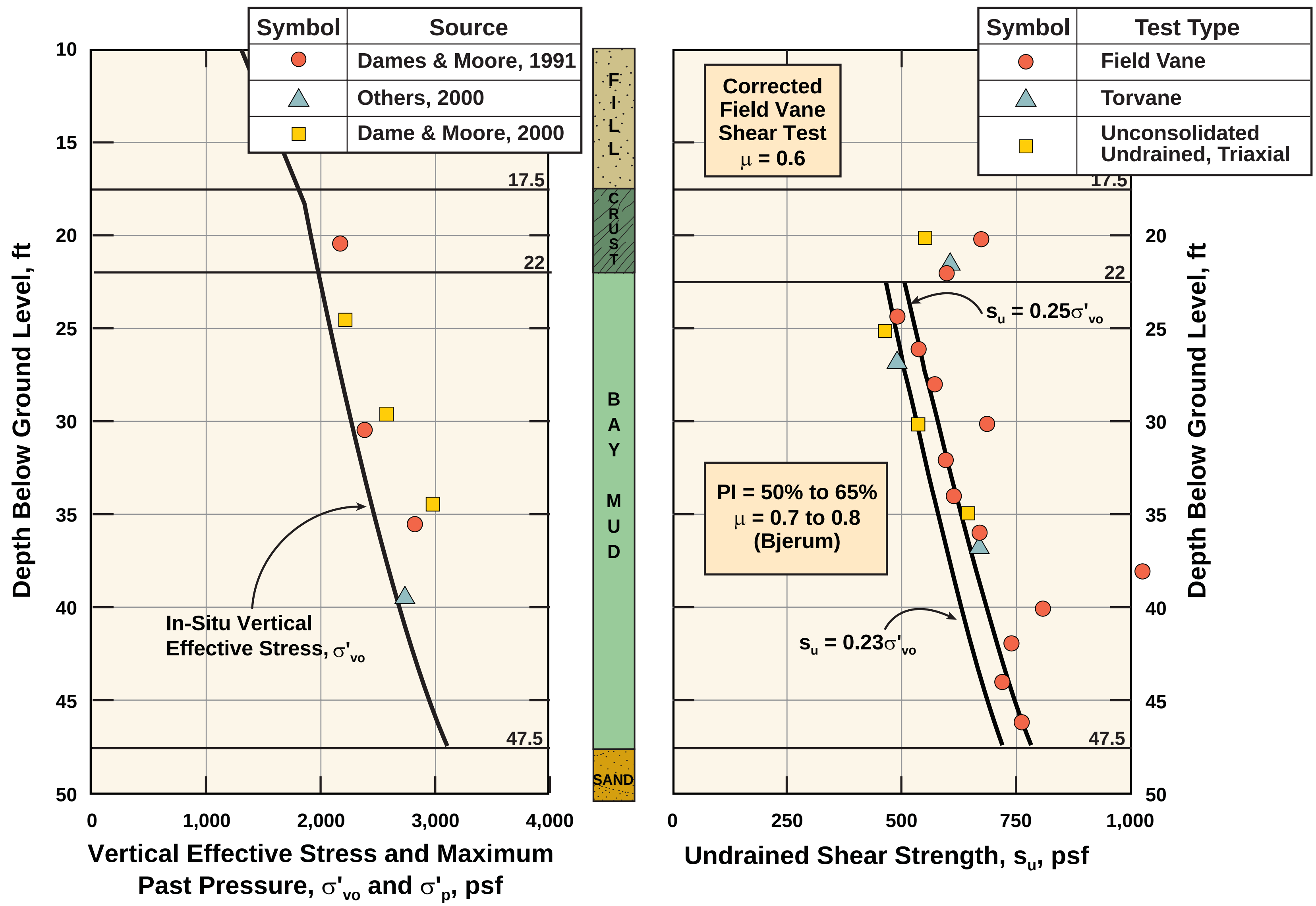


*FIG_40: Stress History and Undrained Vane Shear Strengths of Organic Bay Mud at a Recently Reclaimed Site*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_40*

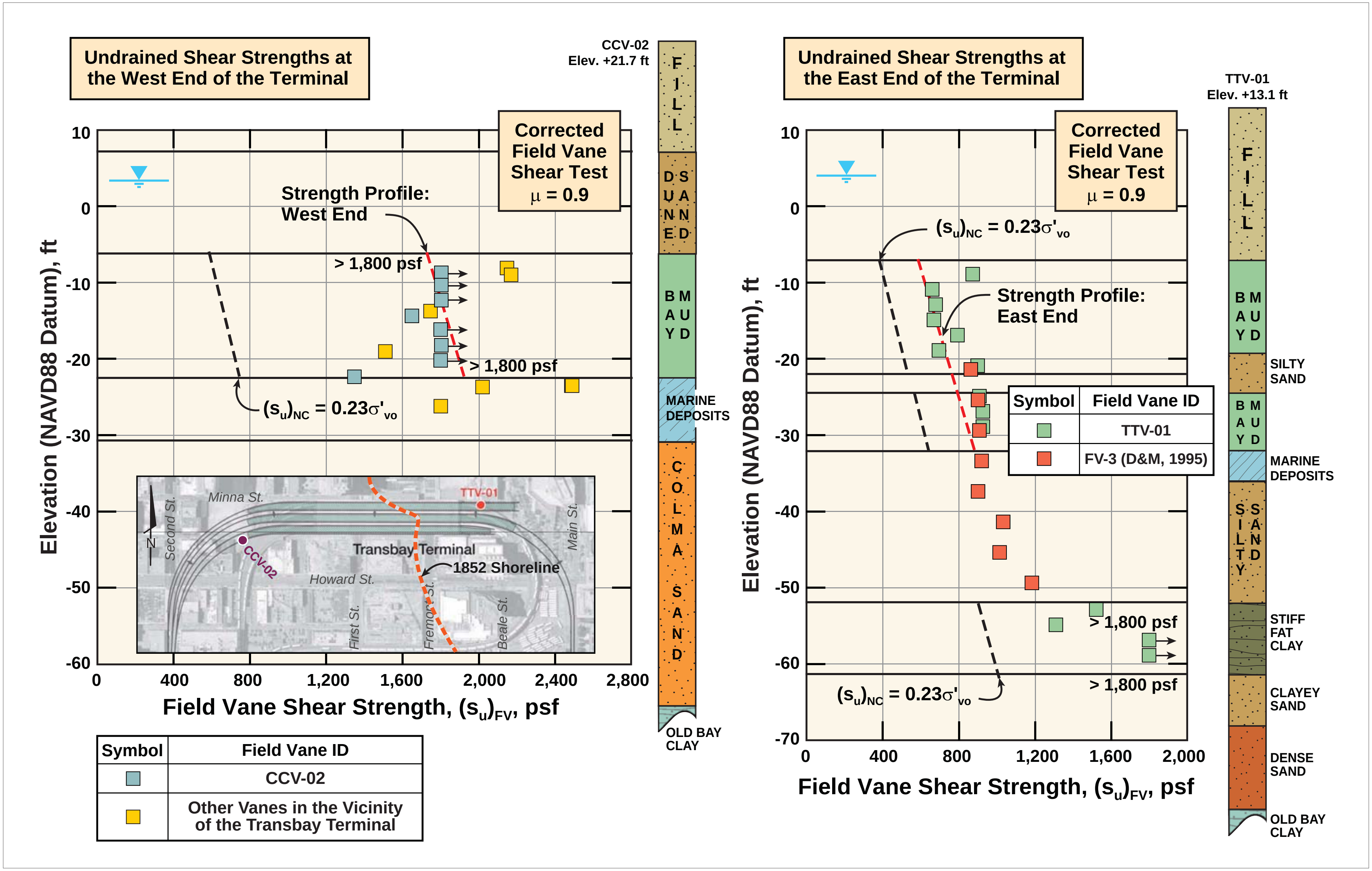


*FIG_41: Undrained Shear Strengths from Vane Tests Variations Between Offshore and Onshore Sites*

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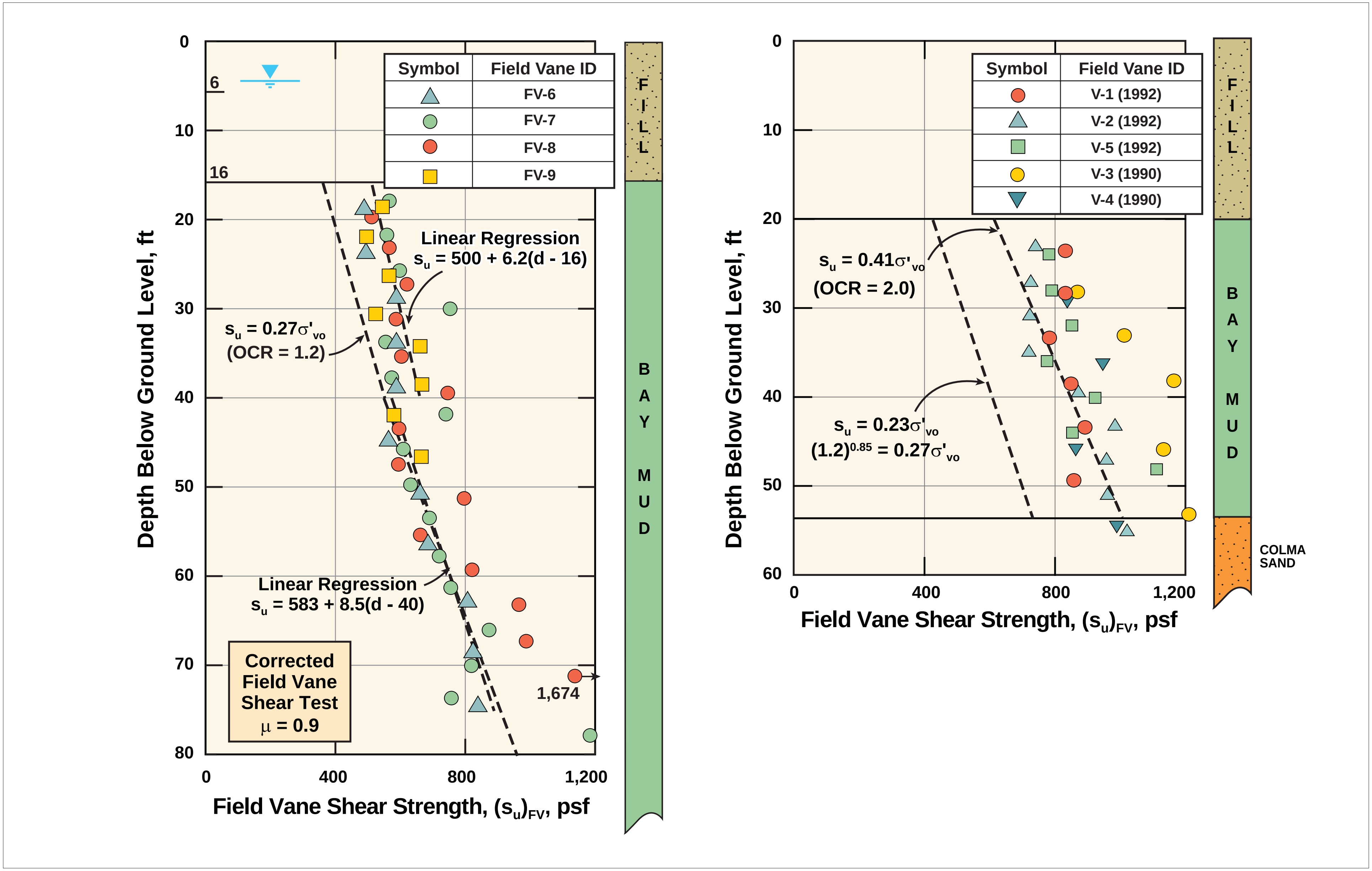


*FIG_42: Field Vane Tests Marshland Area of Islais Creek (Contract C Site)*

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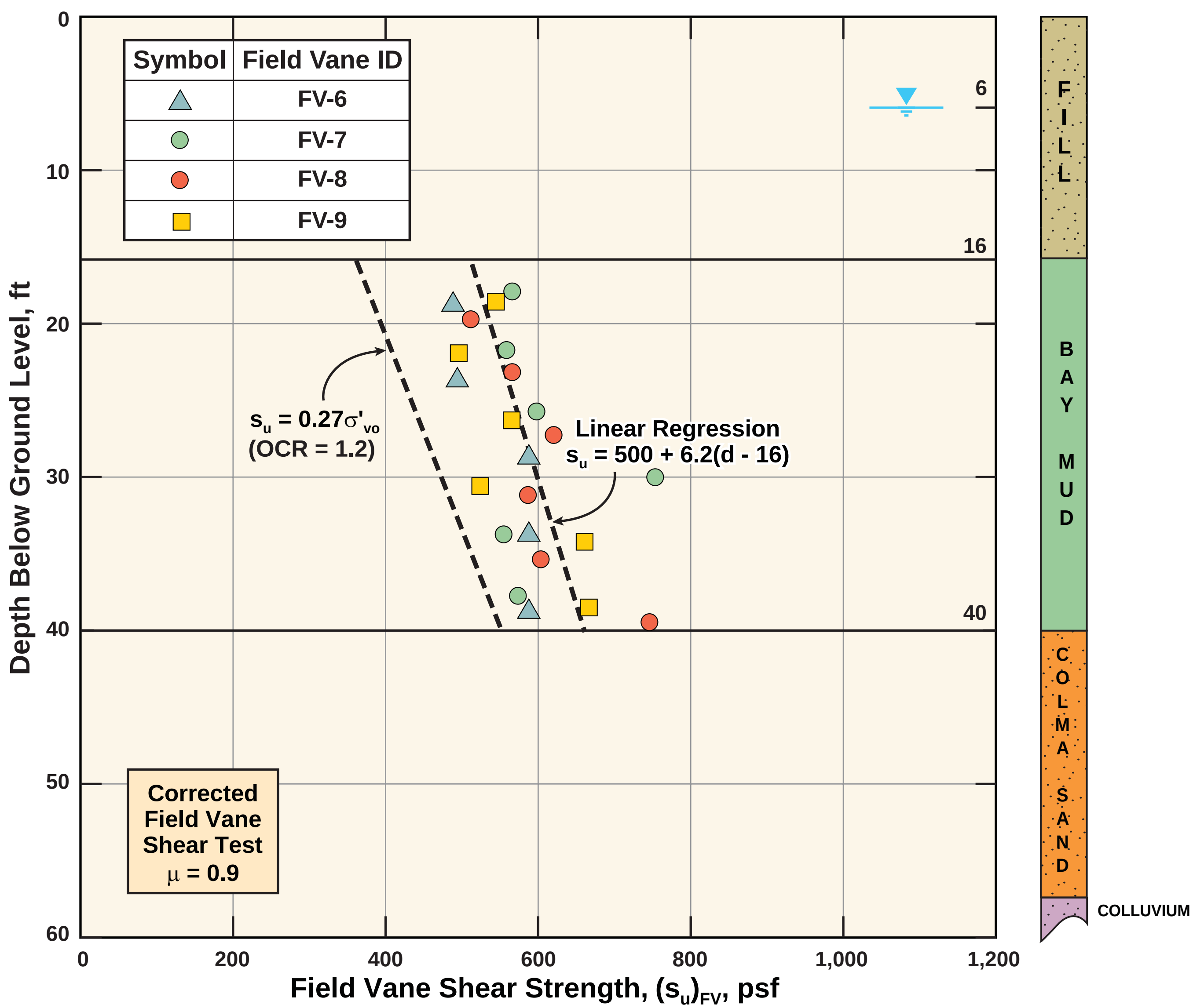


*FIG_43: Comparison of Field Vane Shear Strength with Geotechnical Report Recommendations*

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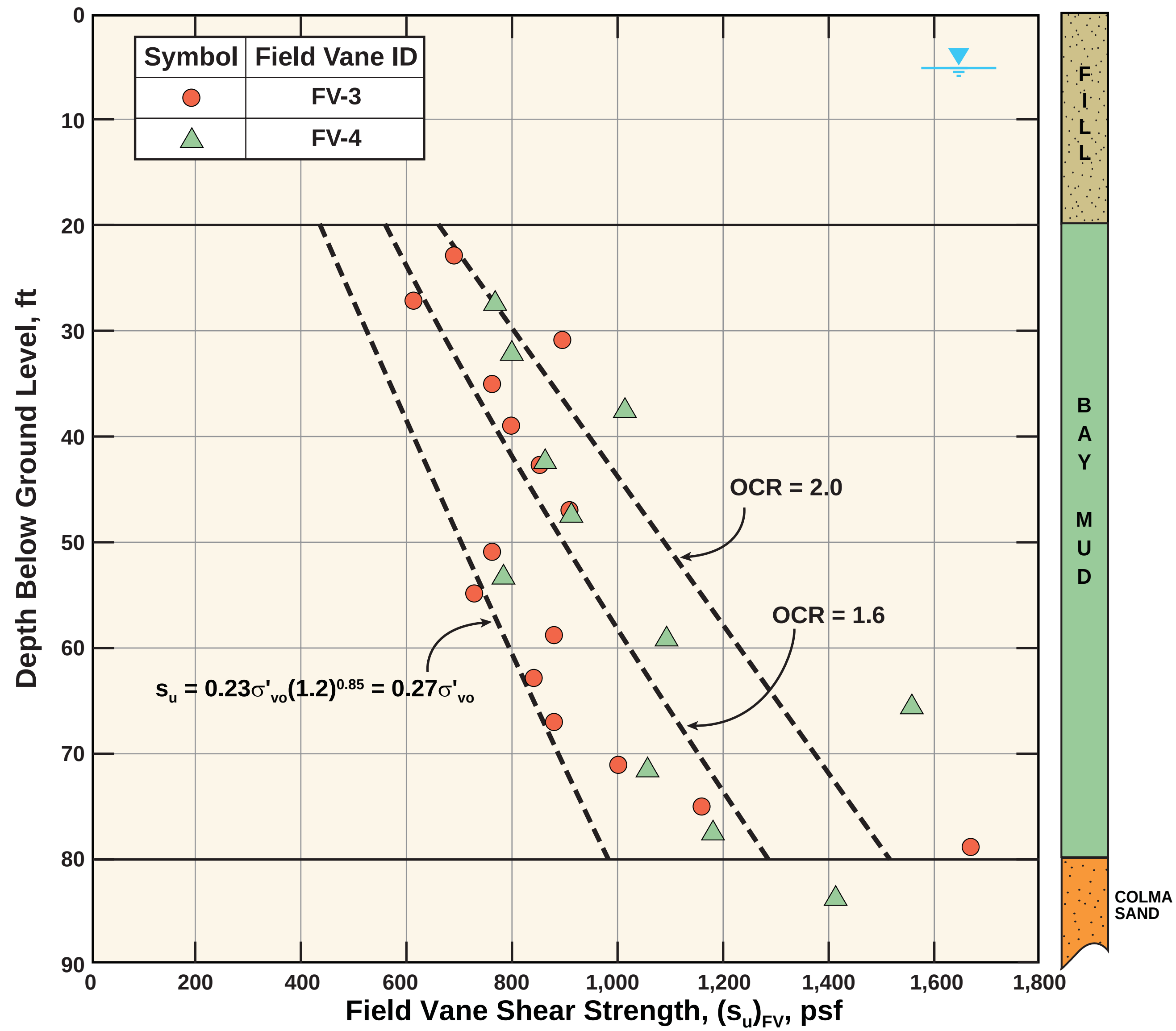


*FIG_44: Variations of Shear Strength Behavior Islais Creek Marshlands – Contract C Near McKinnon Street (Deeper Mud)*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_44*

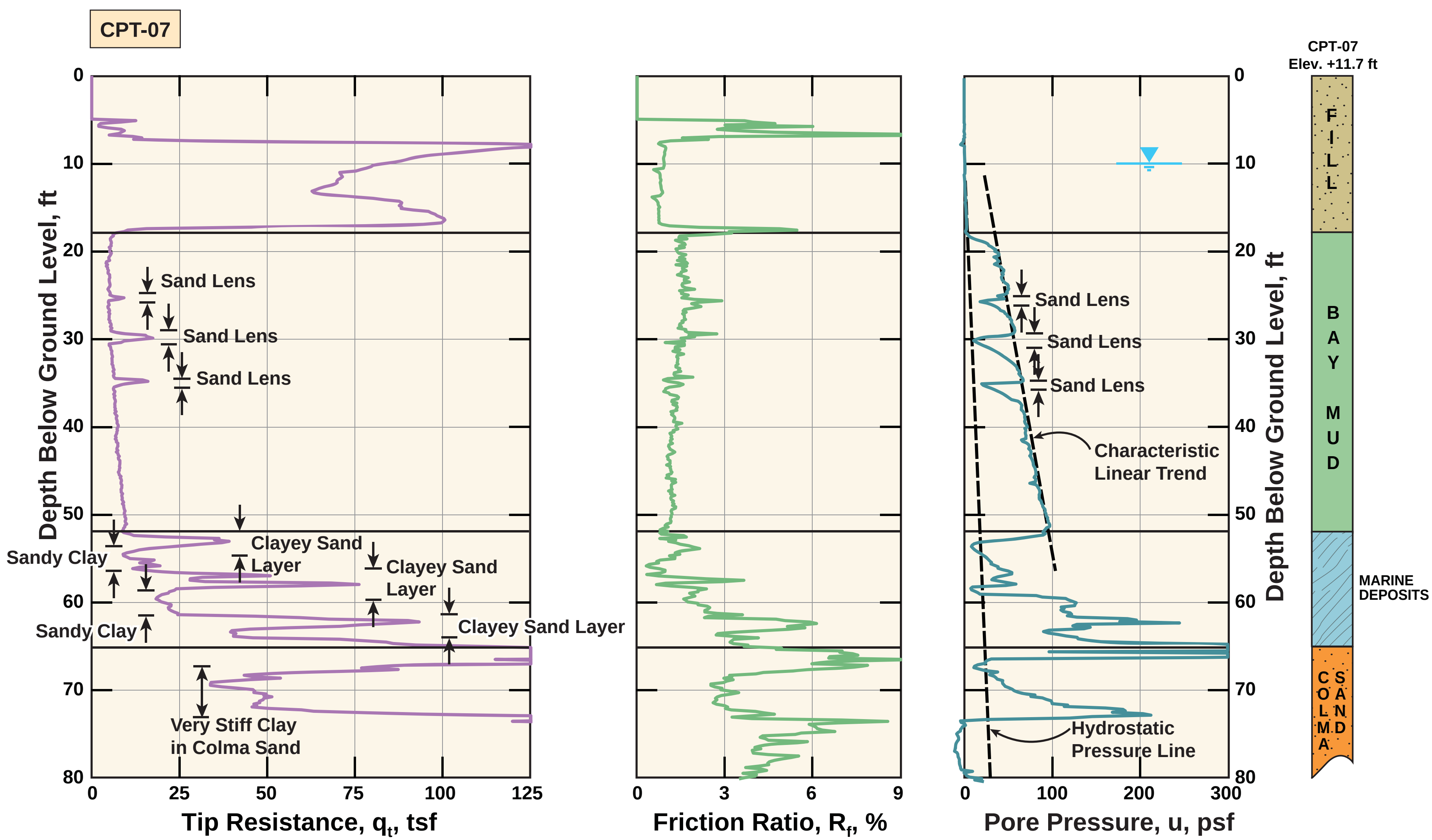


*FIG_45: Typical Results of CPT Tests in Bay Mud*

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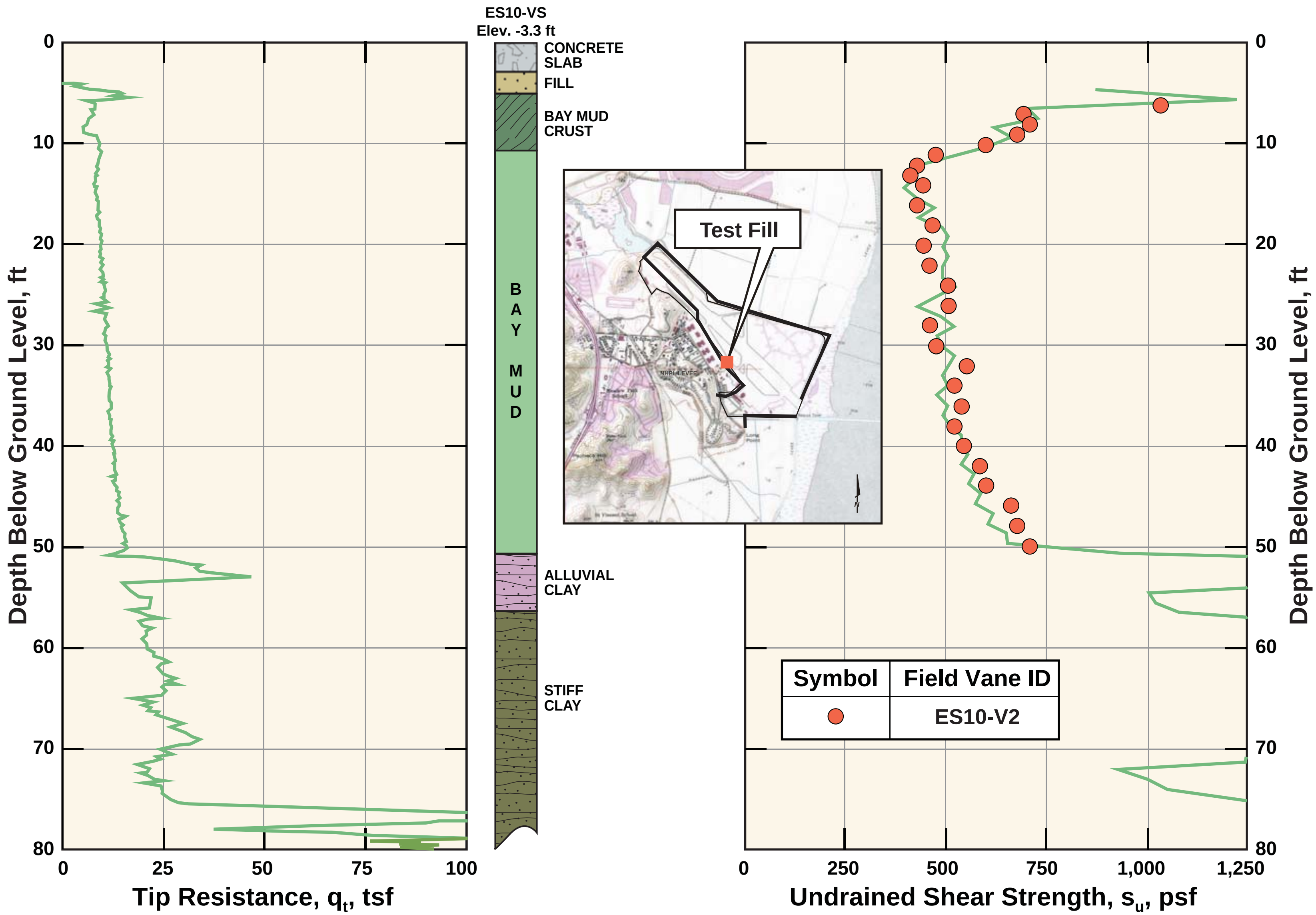


*FIG_46: Correlation of Undrained Strengths from CPT and Vane Shear Tests Hamilton Field Site*

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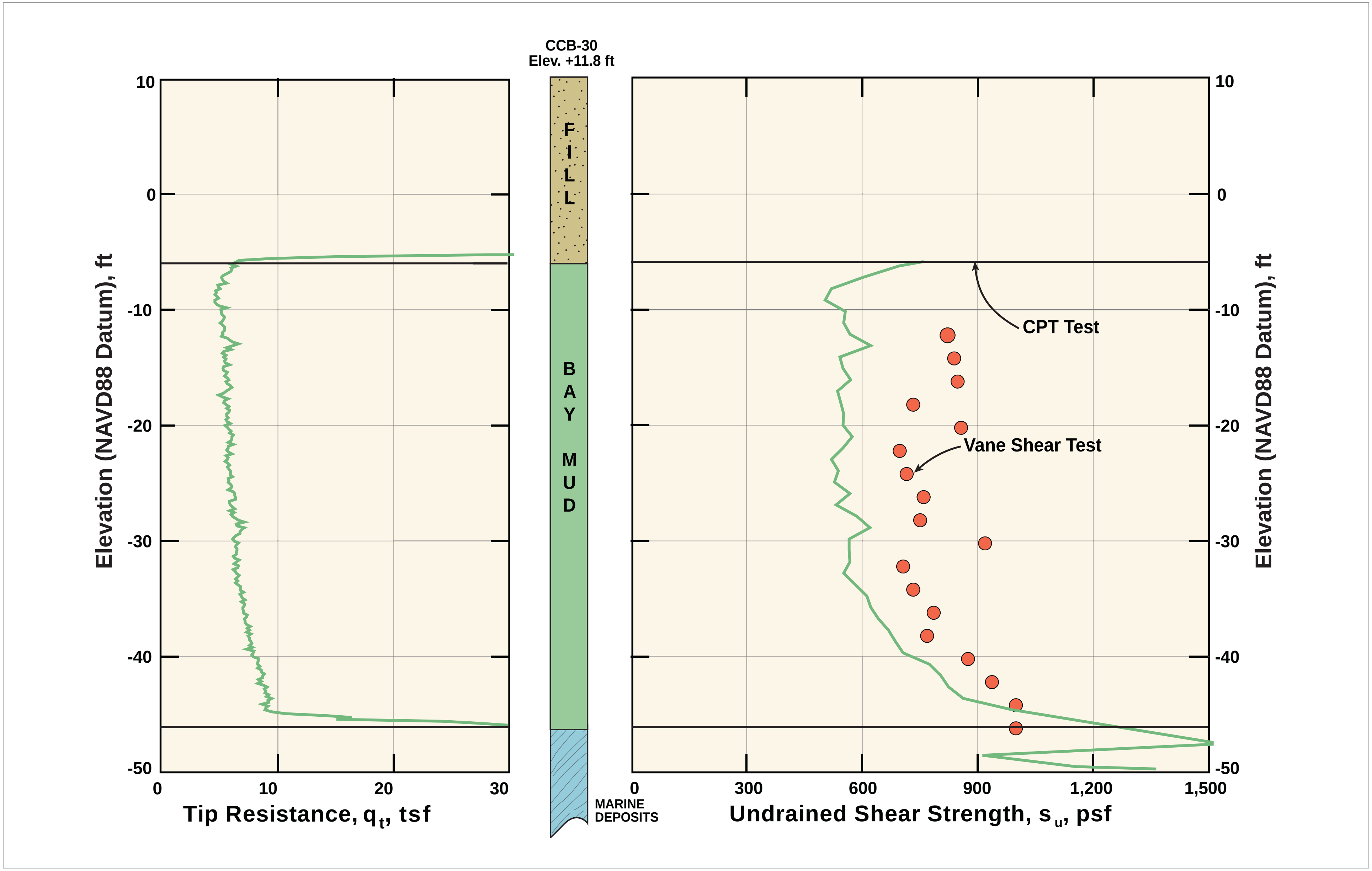


*FIG_47: Correlation of Undrained Strengths from CPT and Vane Shear Tests Mission Bay Site Along Townsend Street*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_47*

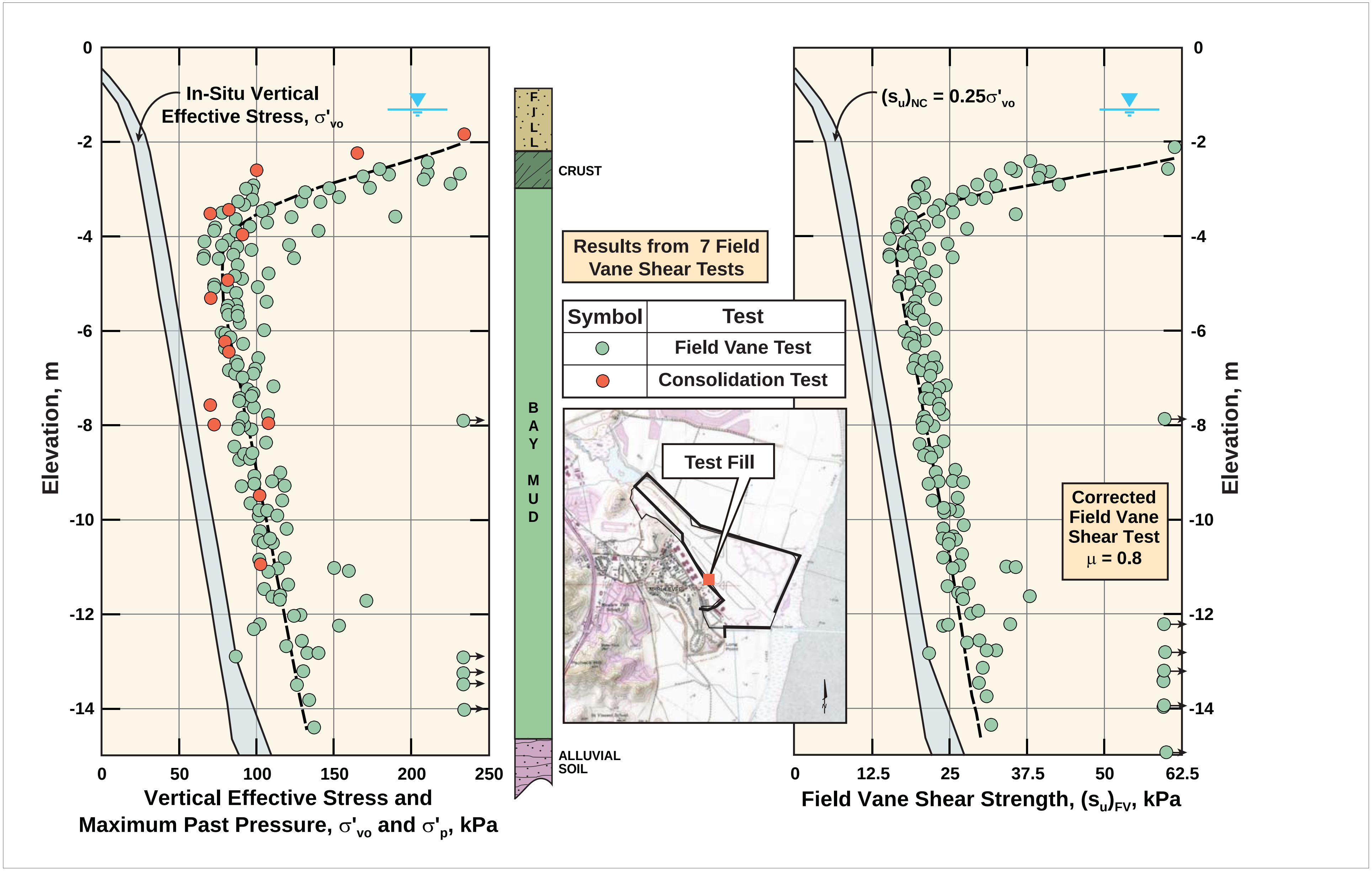


*FIG_48: Estimation of Maximum Past Pressures from Vane Shear Tests Hamilton Field Site*

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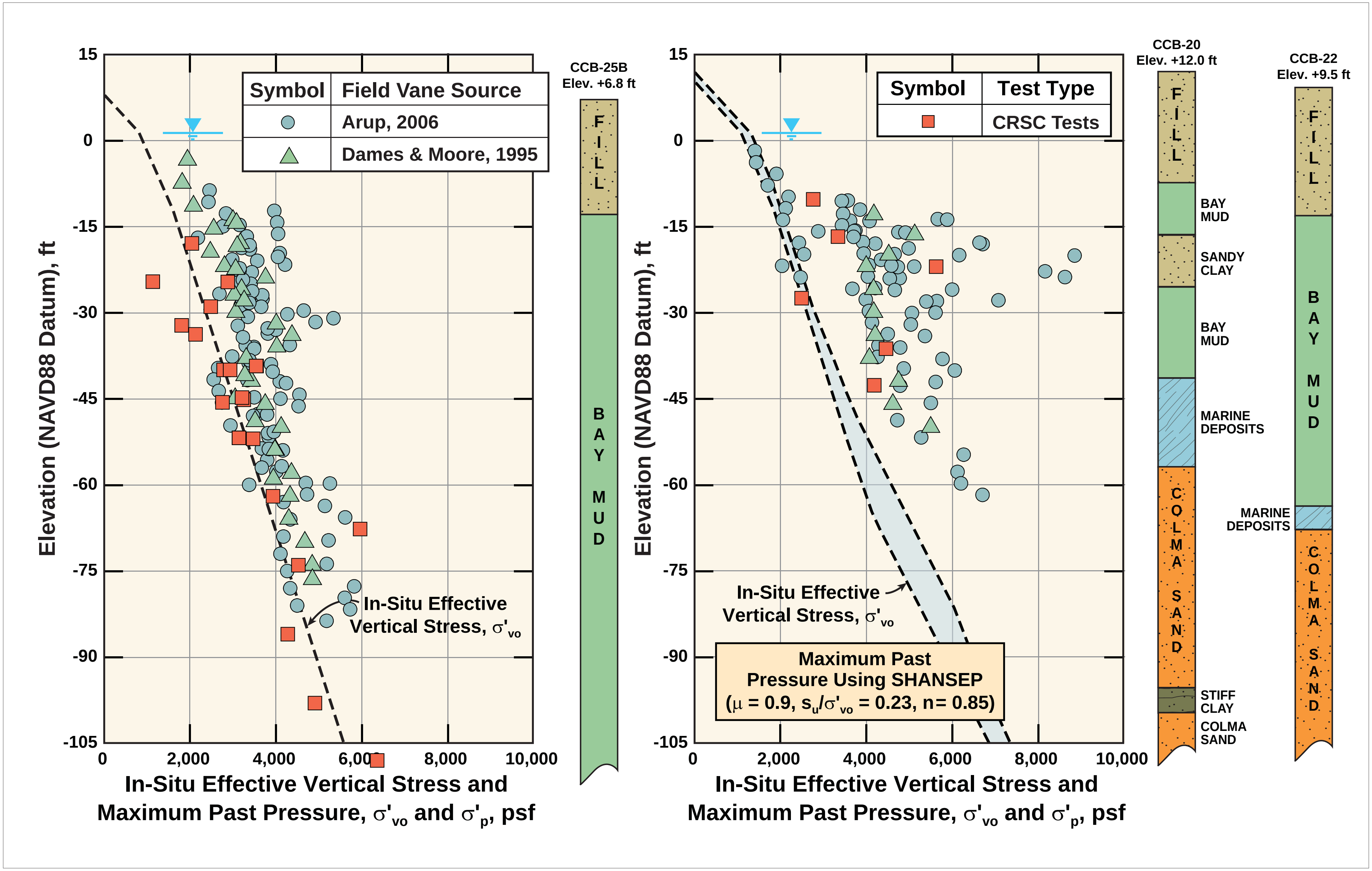


*FIG_49: Stress History of Bay Mud (Maximum Past Pressures)*

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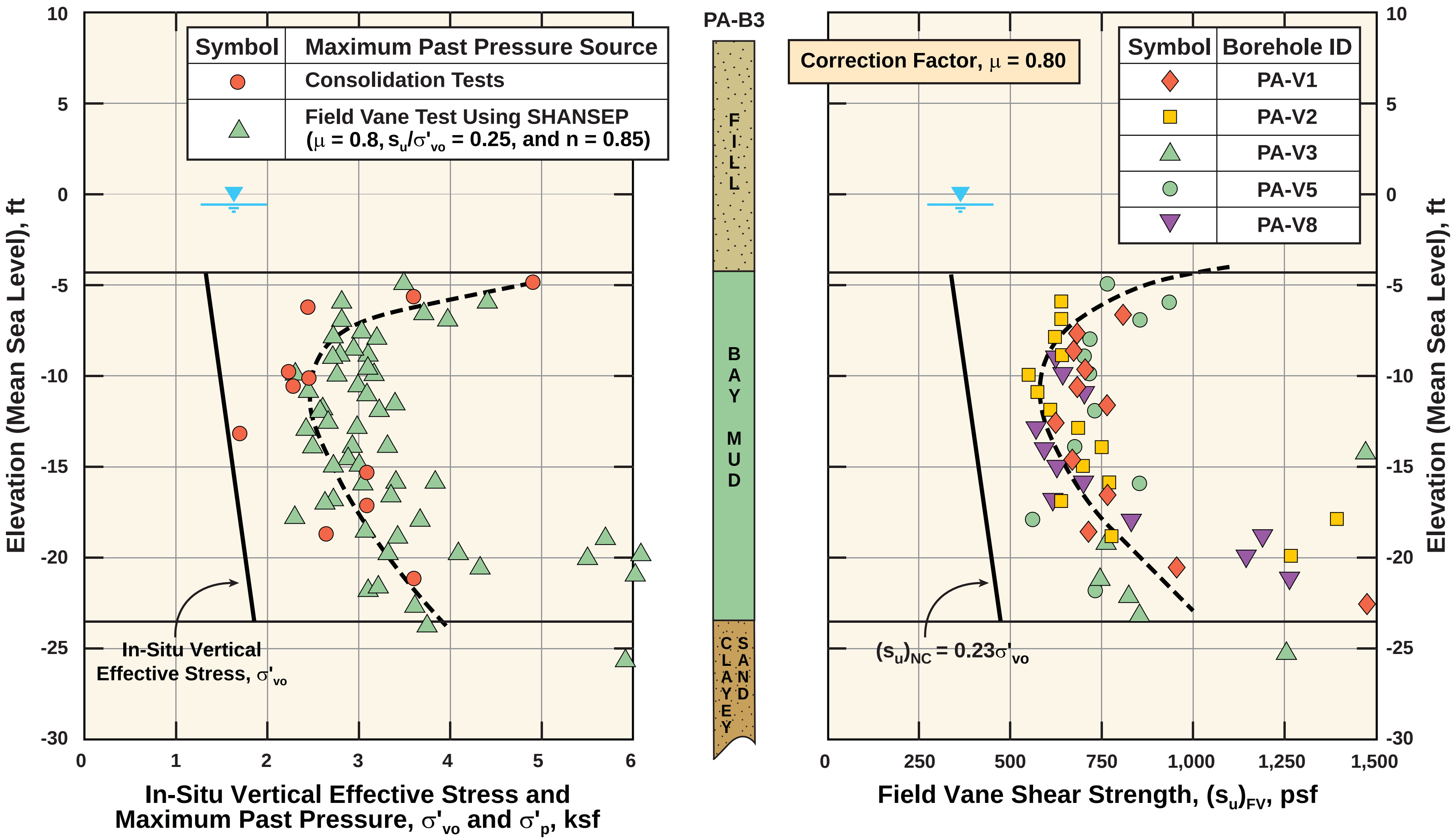


*FIG_50: Estimation of Maximum Past Pressures from Vane Shear Tests Hamilton Field Site: Shallow Mud Area Under Existing Levees*

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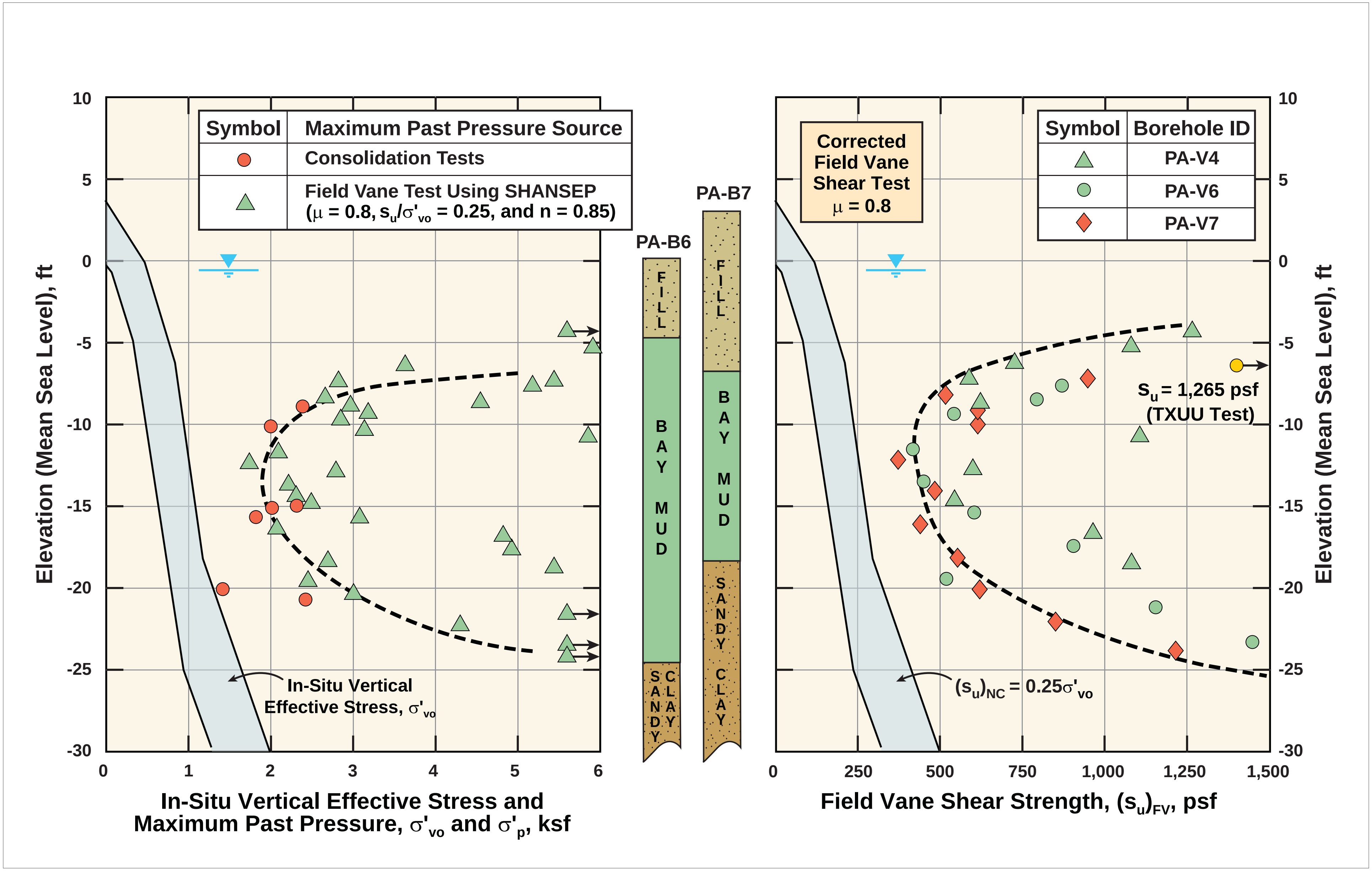


*FIG_51: Estimation of Maximum Past Pressures from Vane Shear Tests Shallow Mud – Free Field Area (HW)*

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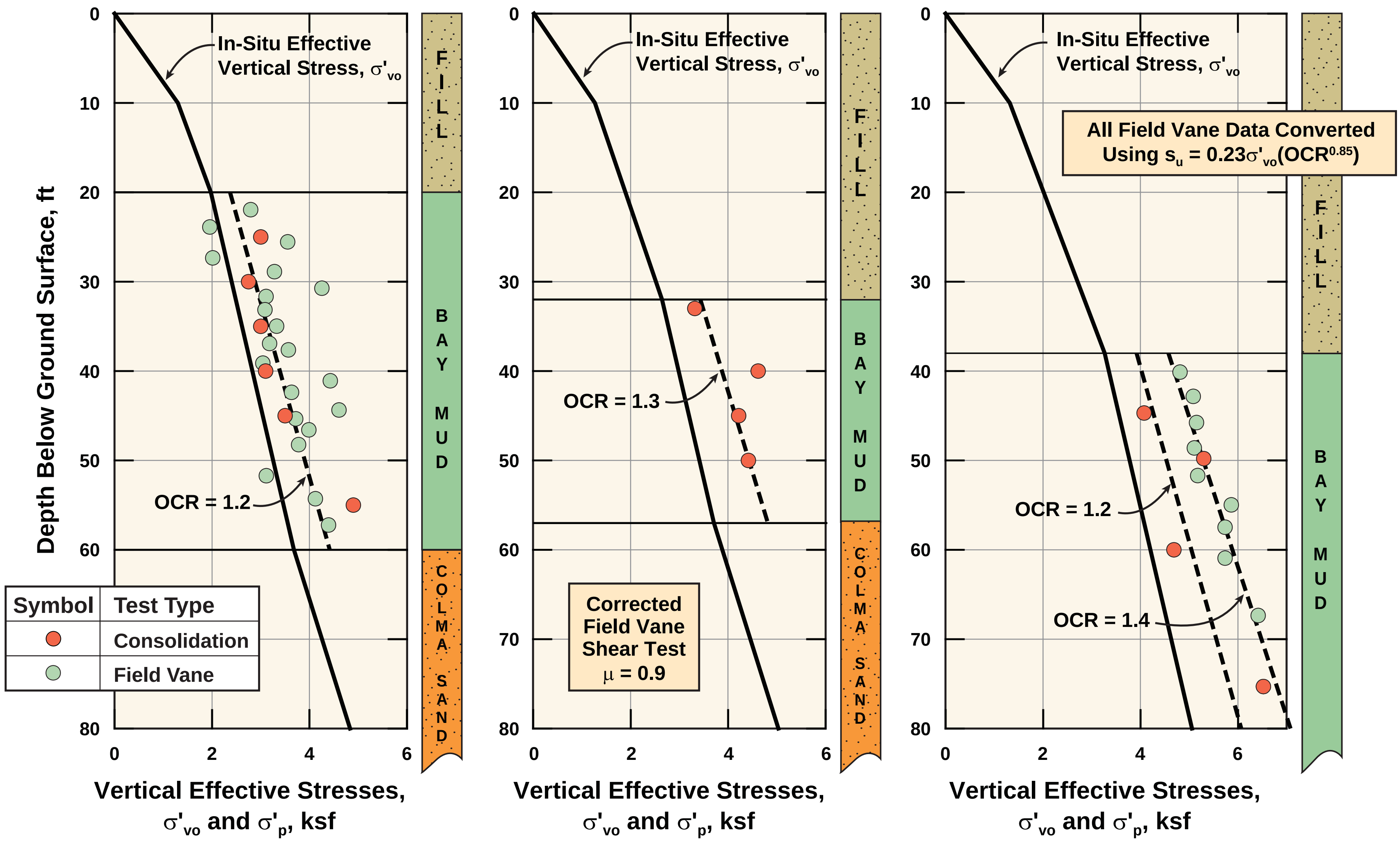


*FIG_52: Stress History for Bay Mud: Muni Metro East*

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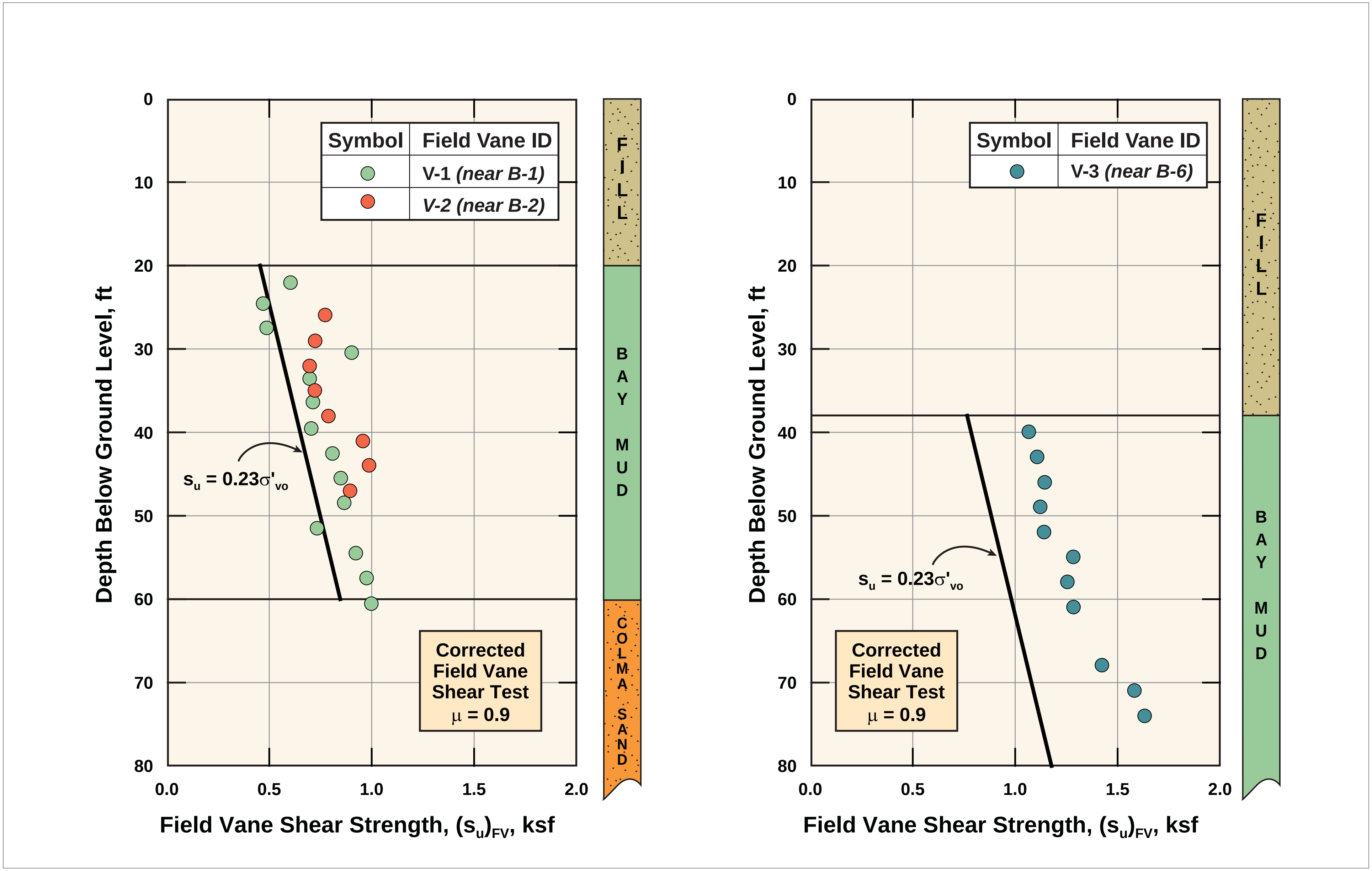


*FIG_52A: Title*

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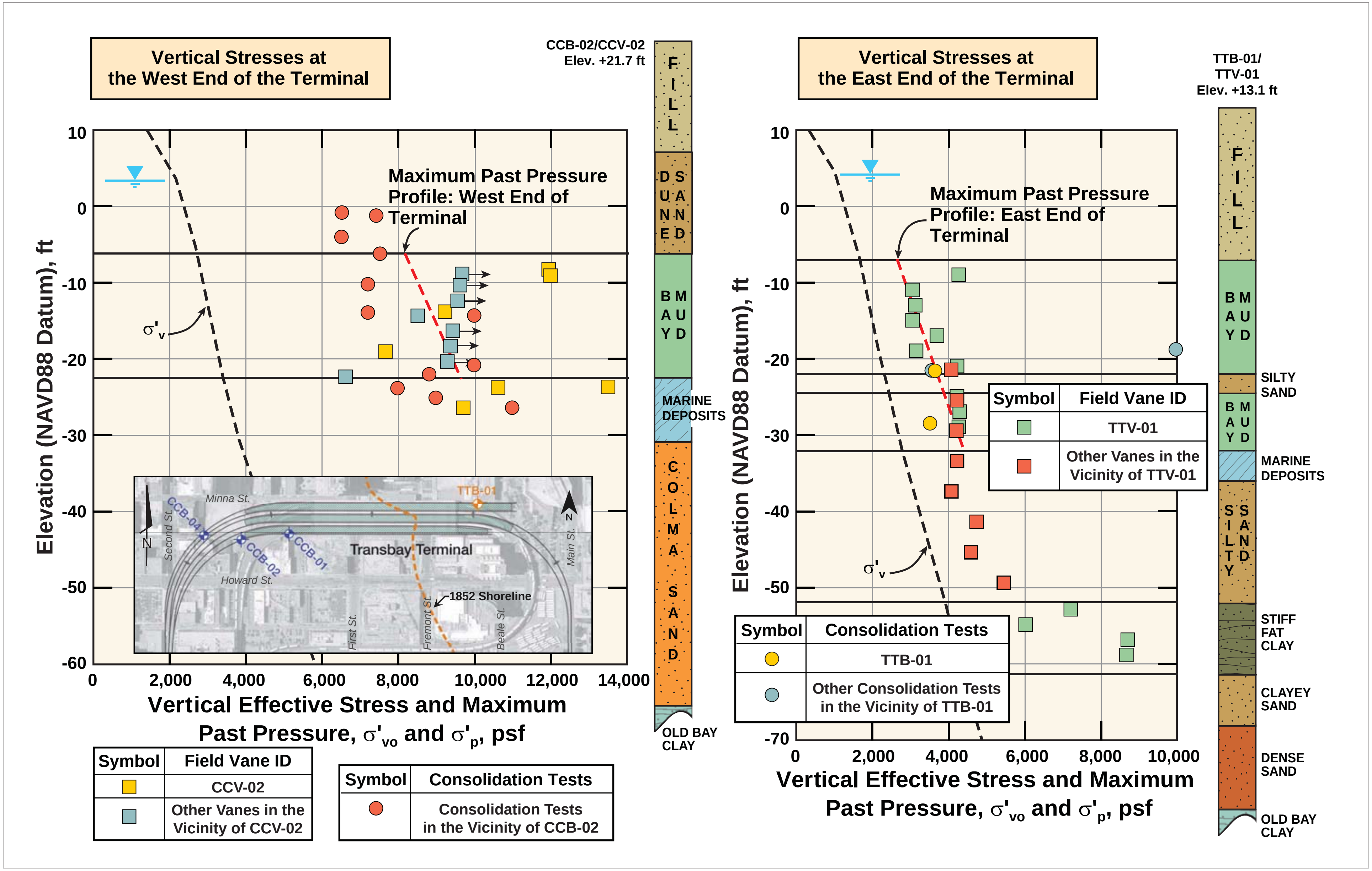


*FIG_53: Estimation of Maximum Past Pressures from Vane Shear Tests Transbay Terminal Site*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_53*

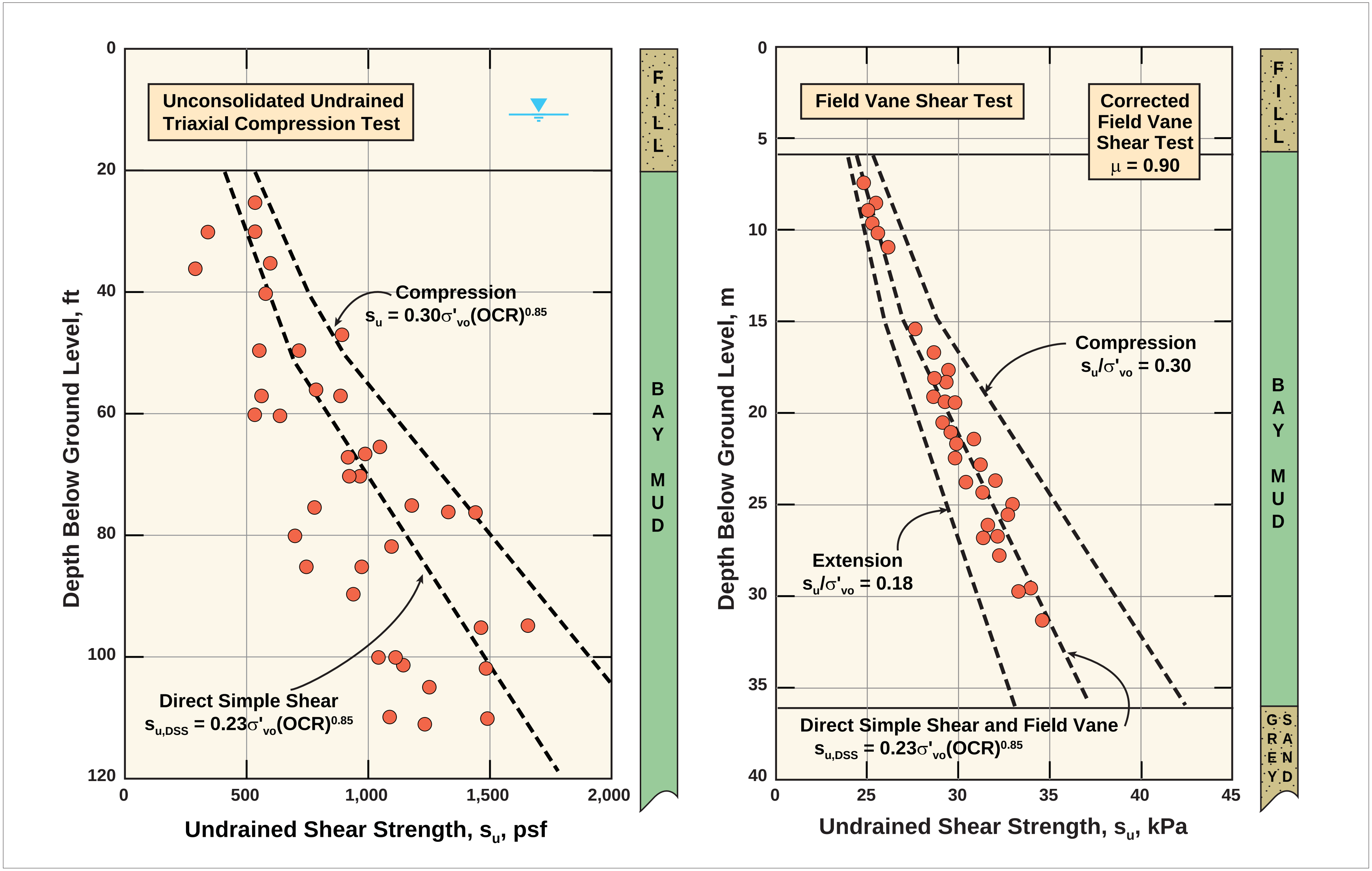


*FIG_54: Undrained Shear Strengths from In-Situ Vane Shear and UU Laboratory Tests Muni Metro Turnback Site*

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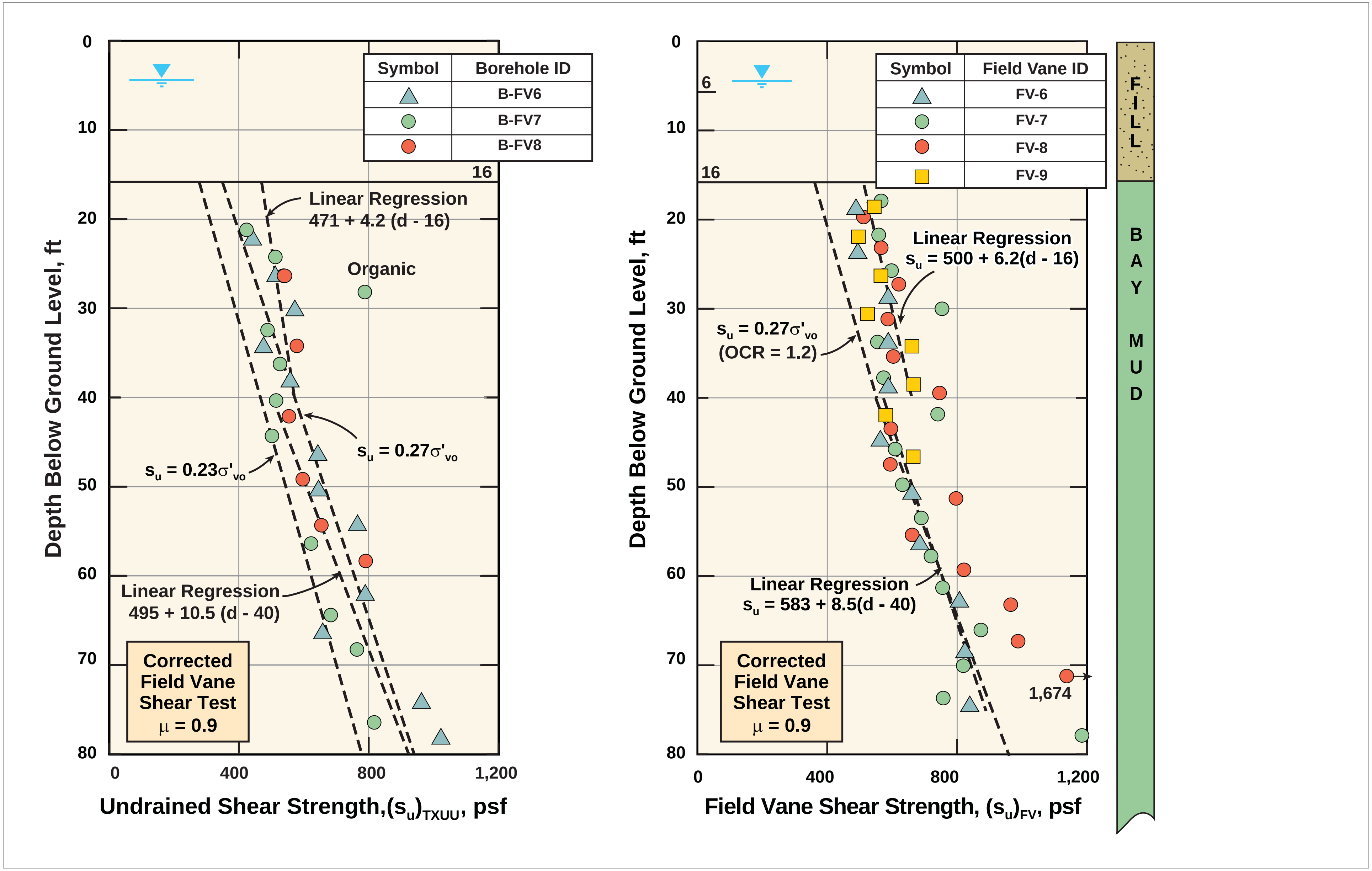



*FIG_54A: Title*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_54A*

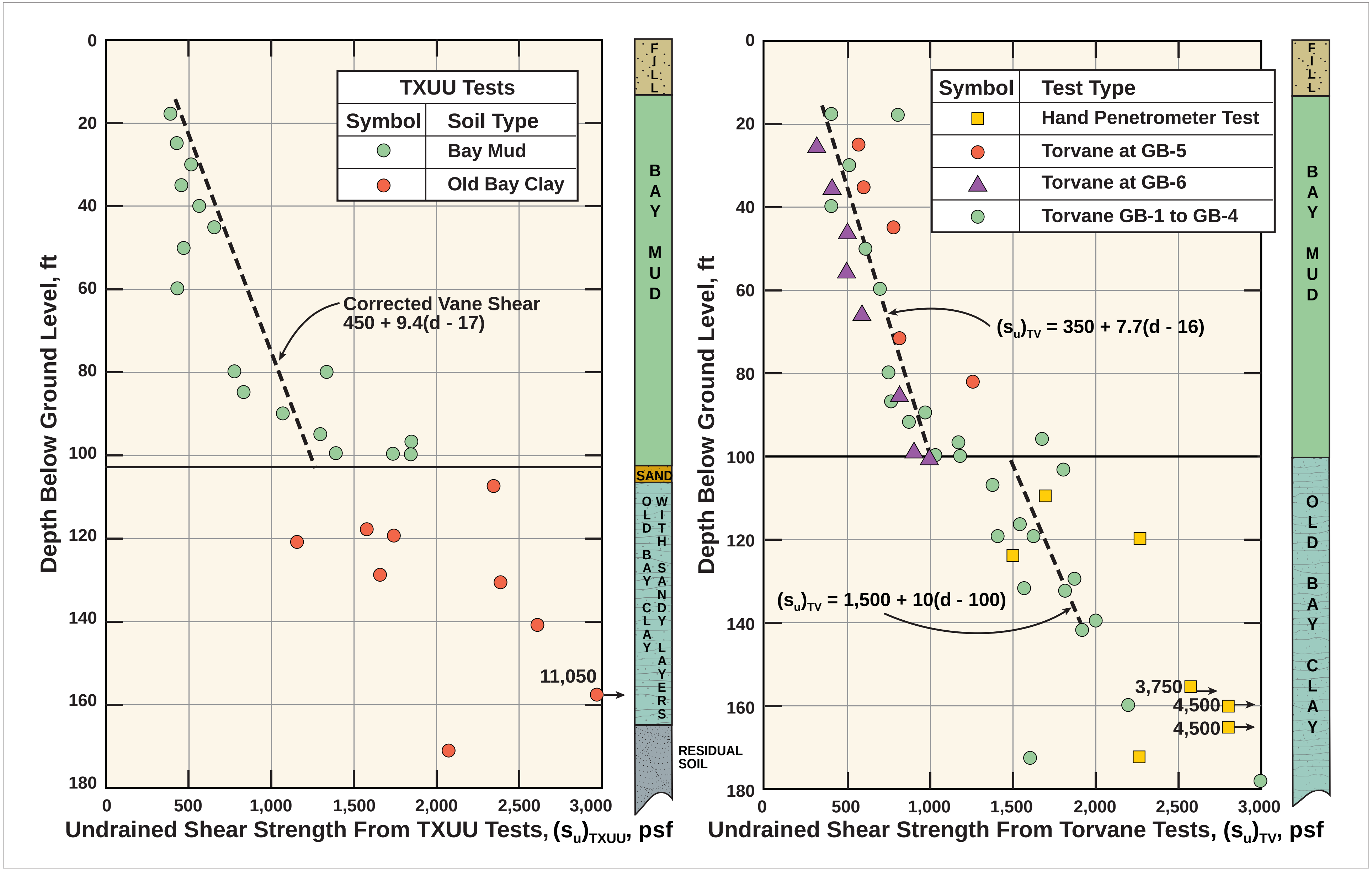



*FIG_55: Undrained Strengths from Vane Shear, UU, and Torvane Tests Digesters Site – Islais Creek Estuary*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_55*

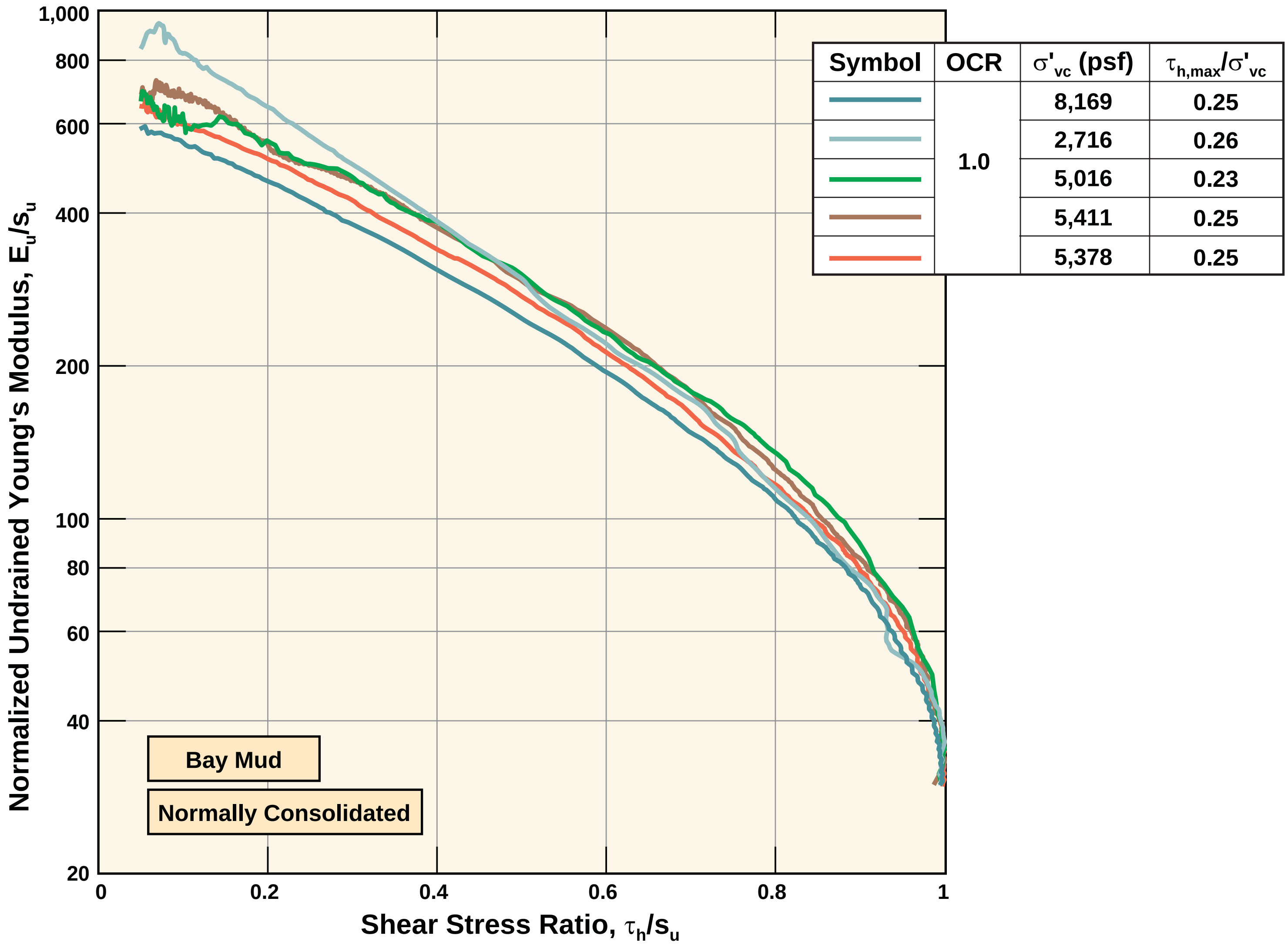



*FIG_56: Variation of Normalized Undrained Modulus with Shear Stress Level DSS Tests on Normally Consolidated Bay Mud*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figure\FIG_56*

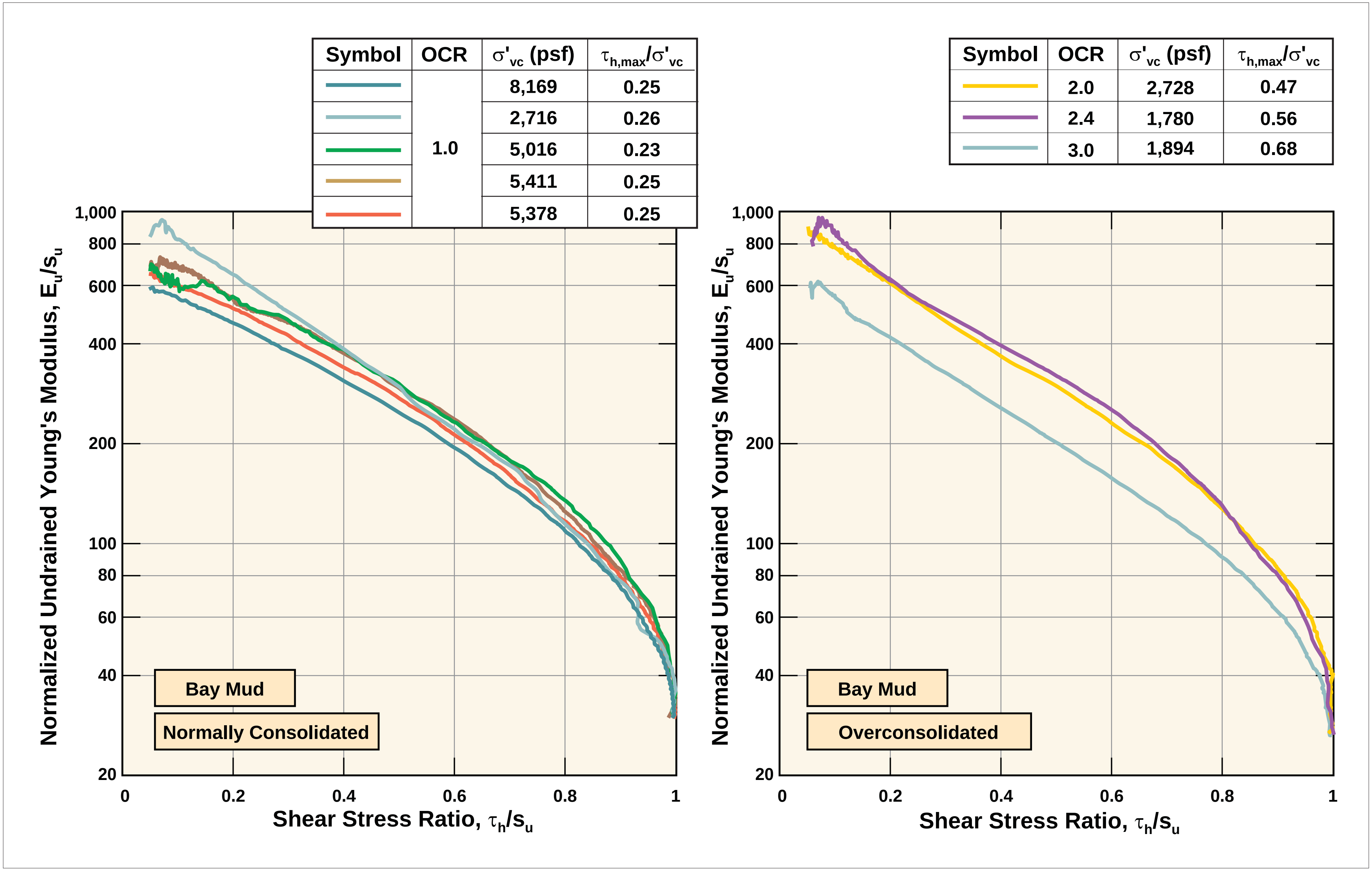



*FIG_57: Normalized Undrained Modulus from DSS Tests Normally and Overconsolidated Bay Mud*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figure\FIG_57*

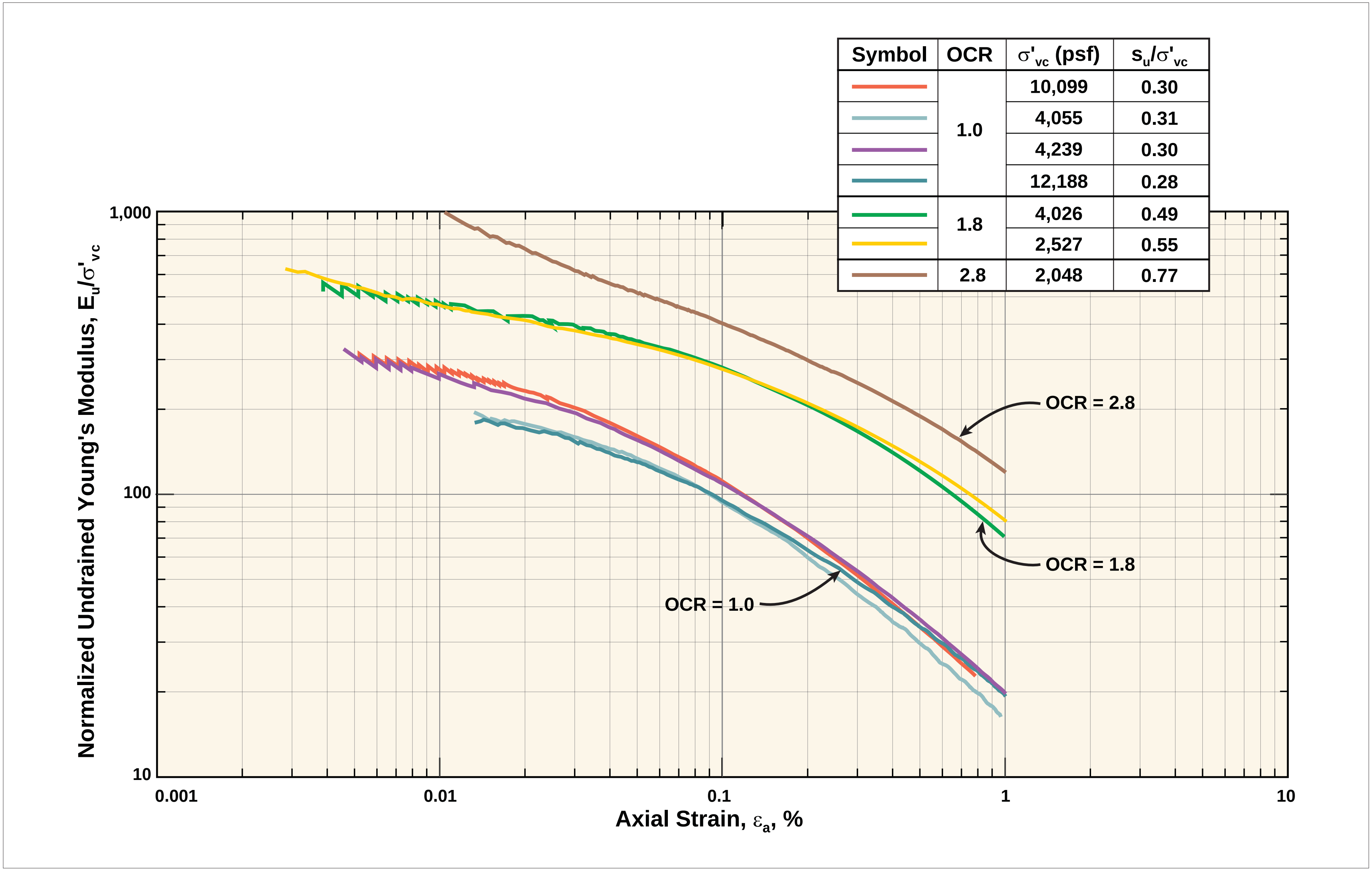



*FIG_58: Variation of Normalized Undrained Modulus with Strain Level From Ko-Triaxial Compression Tests*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figure\FIG_58*

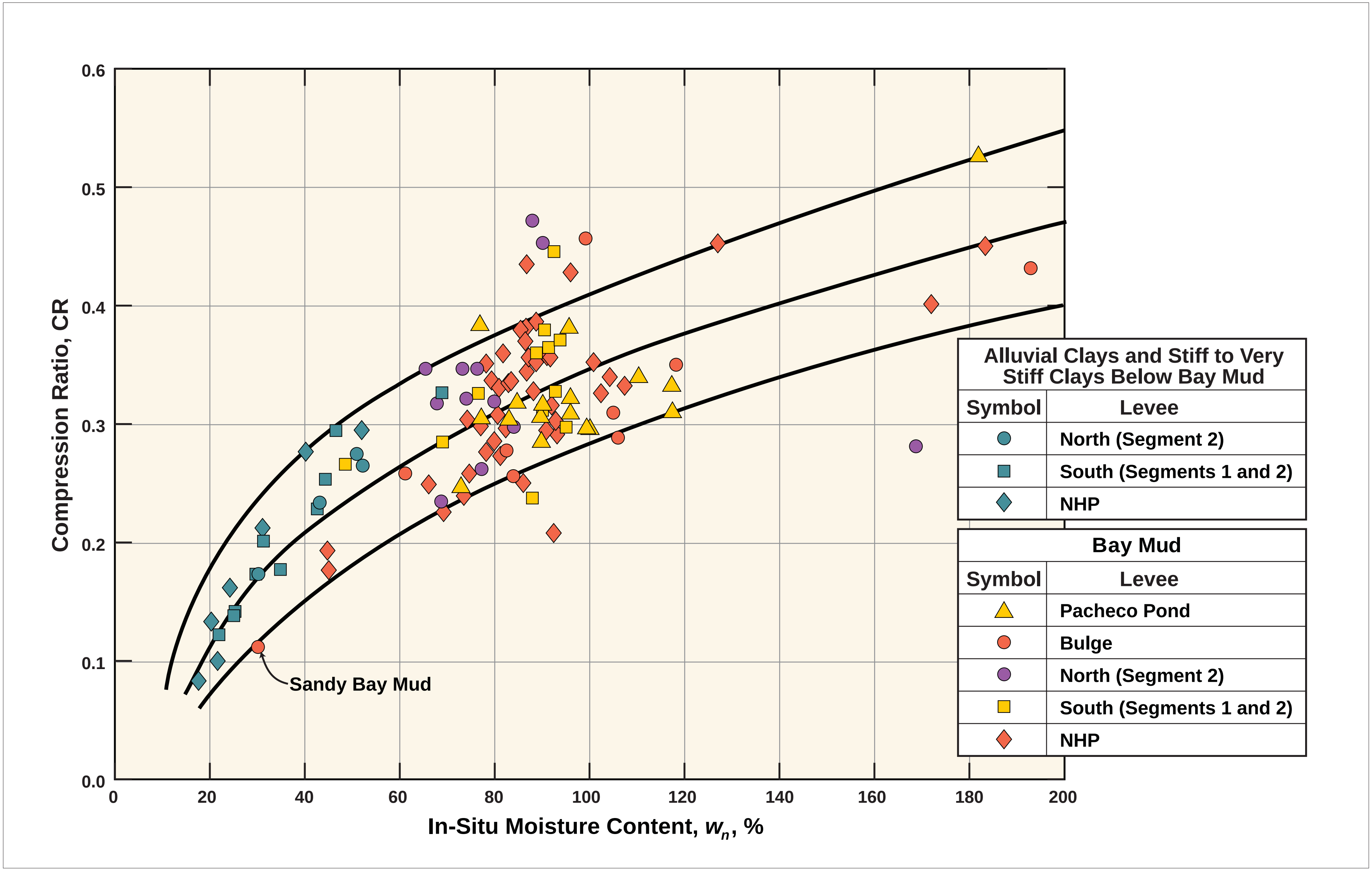



*FIG_59: Variation of Compression Ratio with Moisture Content: Bay Mud and Stiff Clays Hamilton F*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\FIG_59*

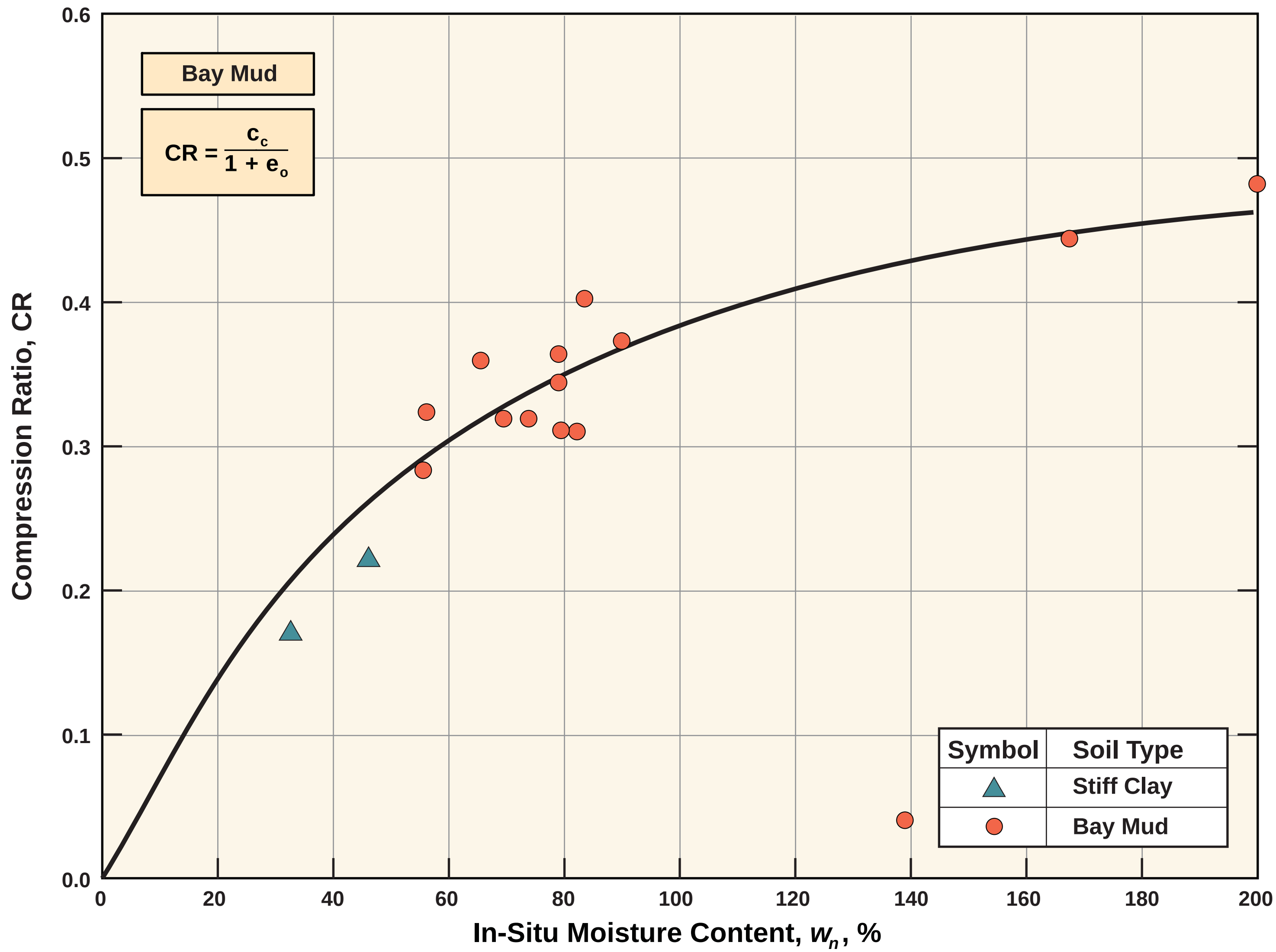



***FIG_60: Variation of Compression Ratio with Moisture Content North Bay Site***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_60*

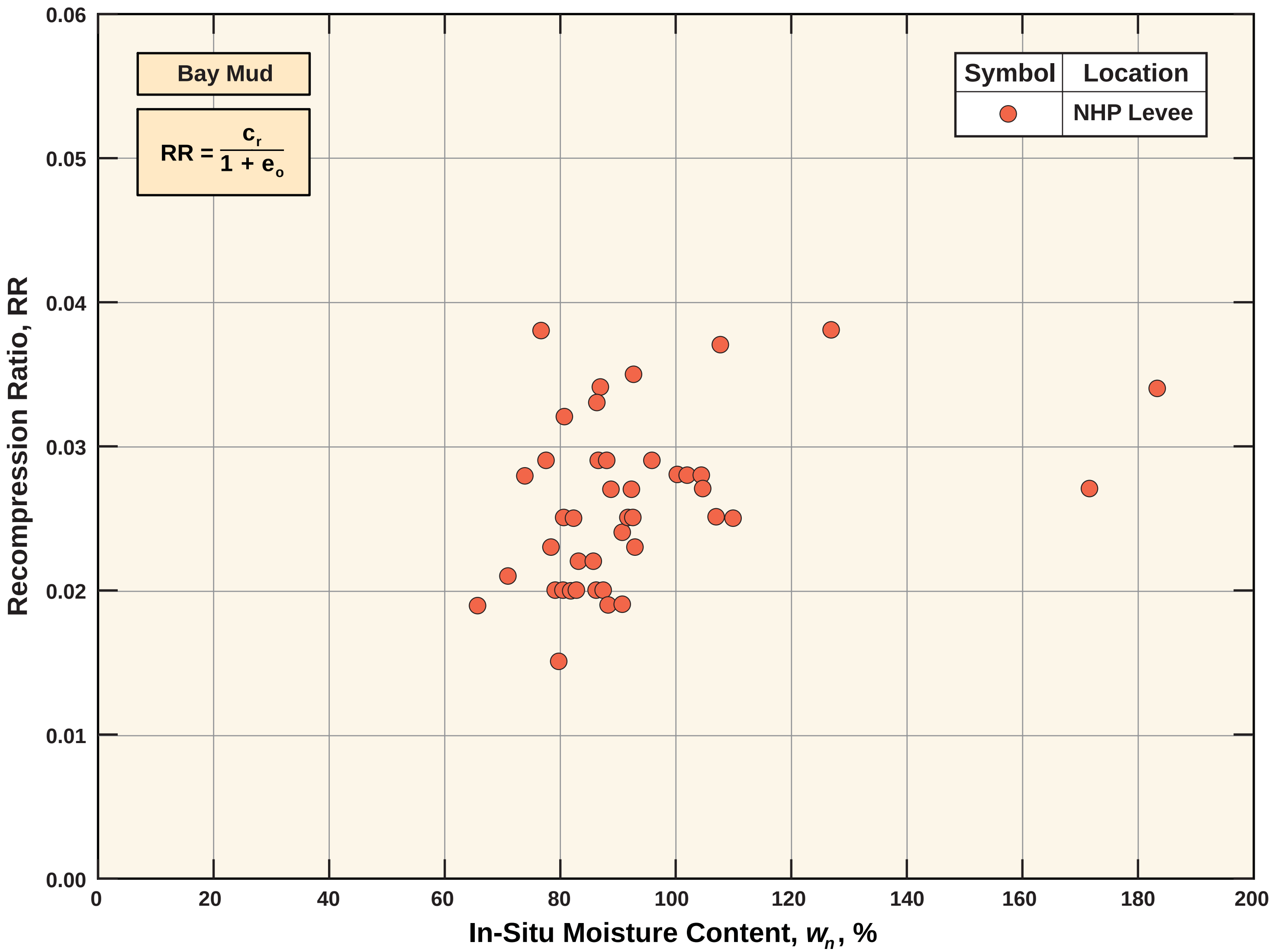



***FIG_61: Variation of Recompression Ratio with Moisture Content***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_61*

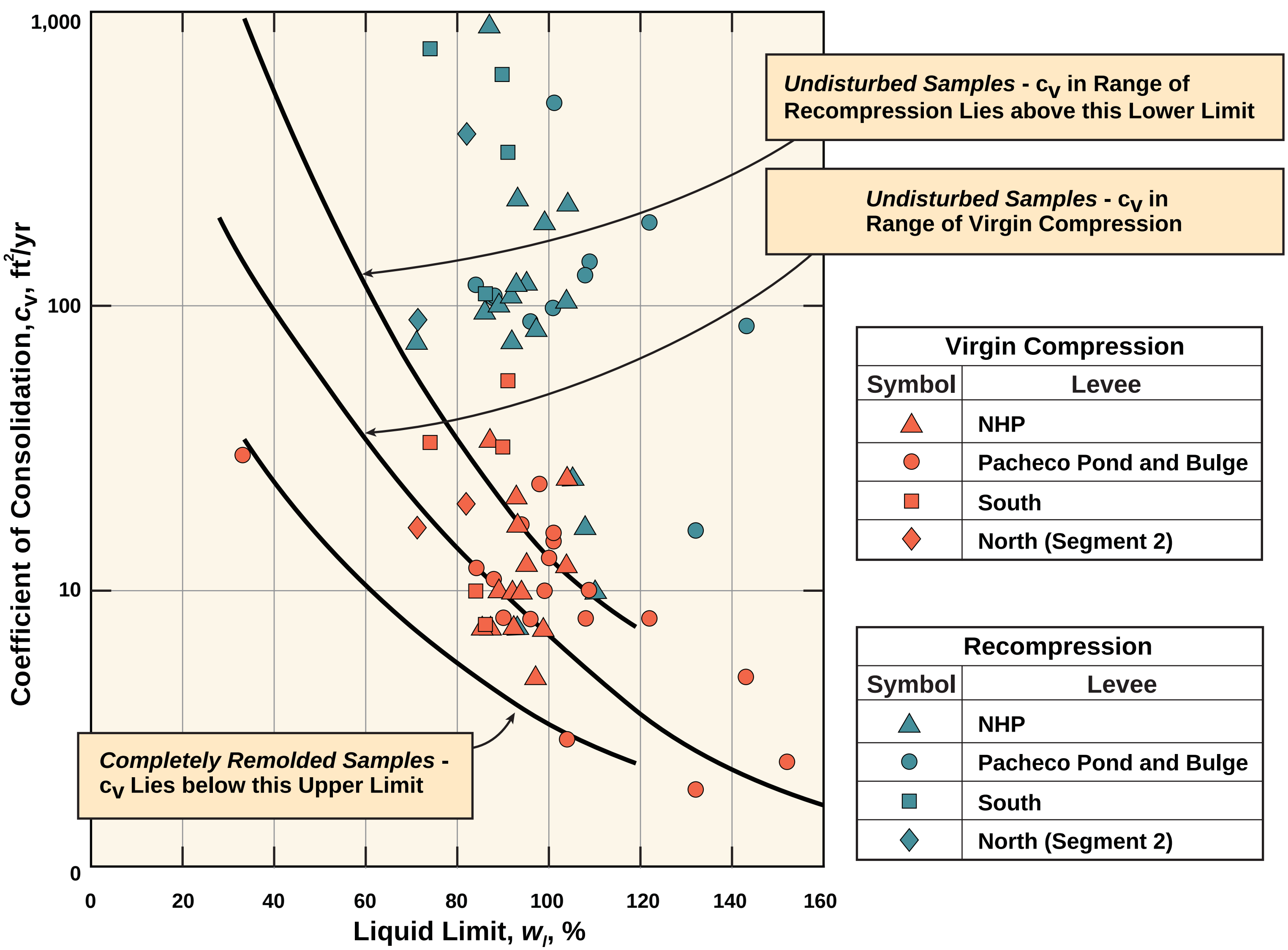



*FIG_62: Variation of Coefficients of Vertical Consolidation with Liquid Limit*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\FIG_62*

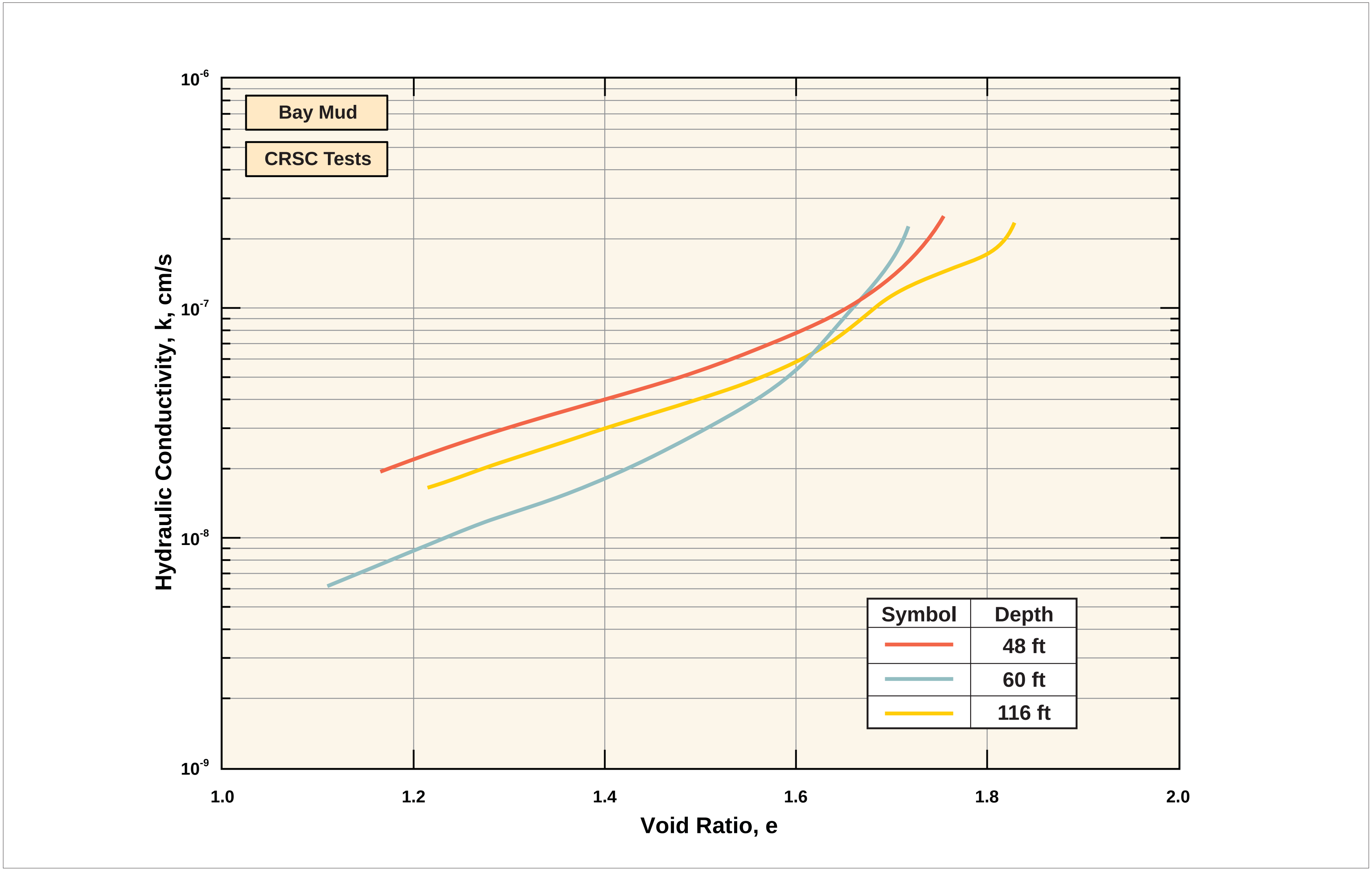



***FIG_62A: Variation of Hydraulic Conductivity with Void Ratio***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_62A*

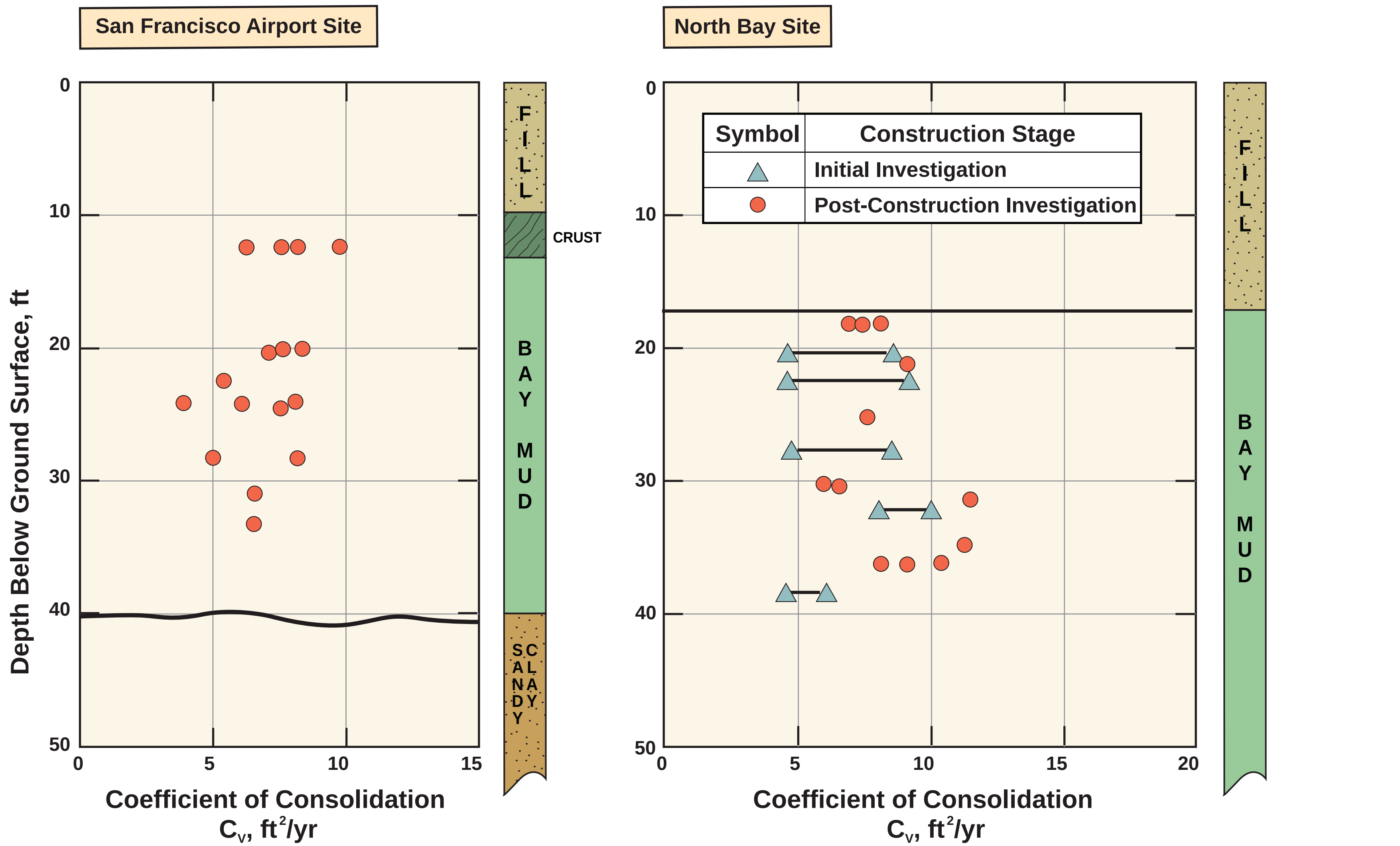



*FIG_63: Coefficient of Vertical Consolidation of Organic Bay Mud*

W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\FIG_63

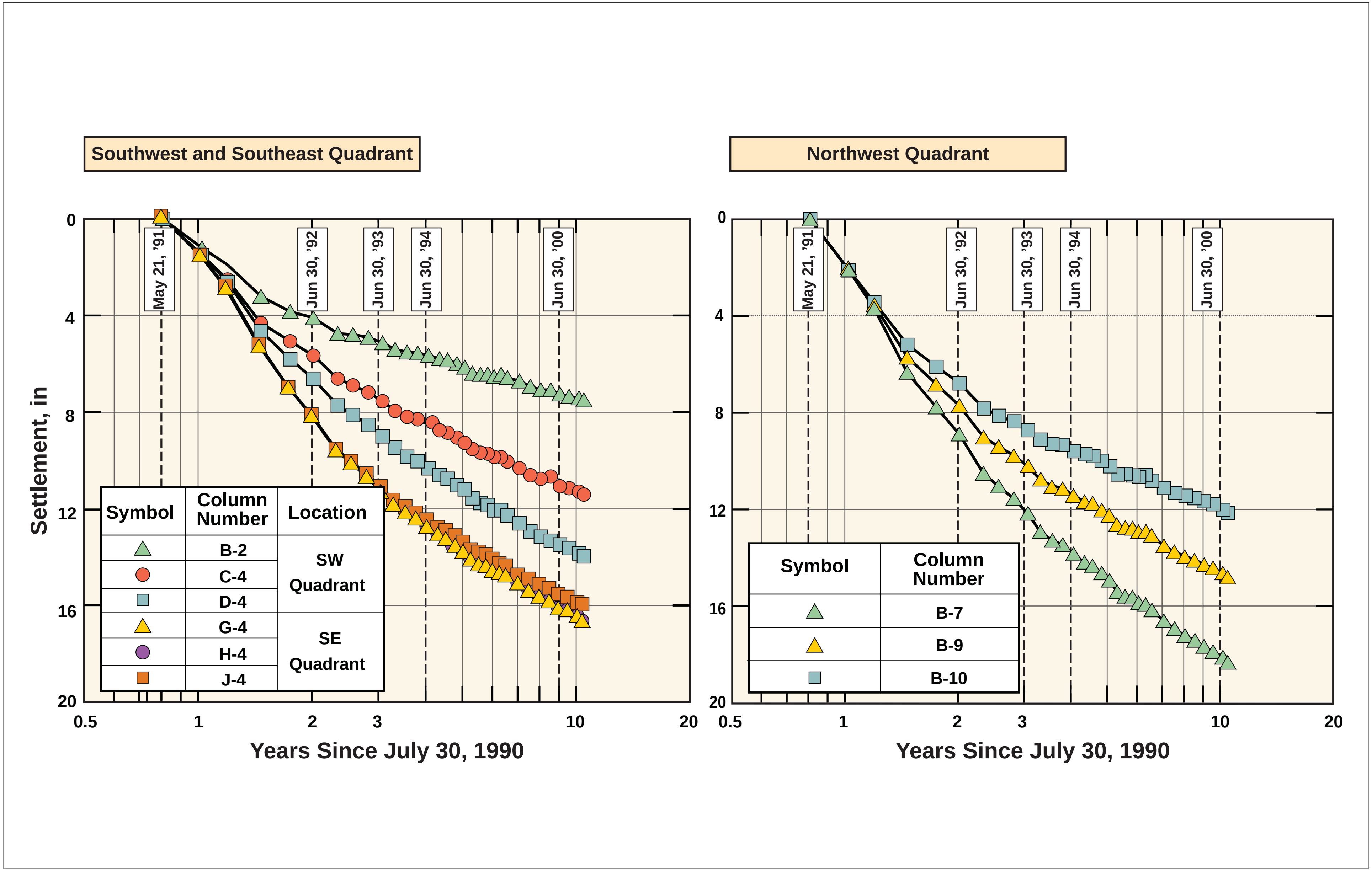



***FIG_64: Post-Construction Settlements at a Preloaded Site North Bay Site***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_64*

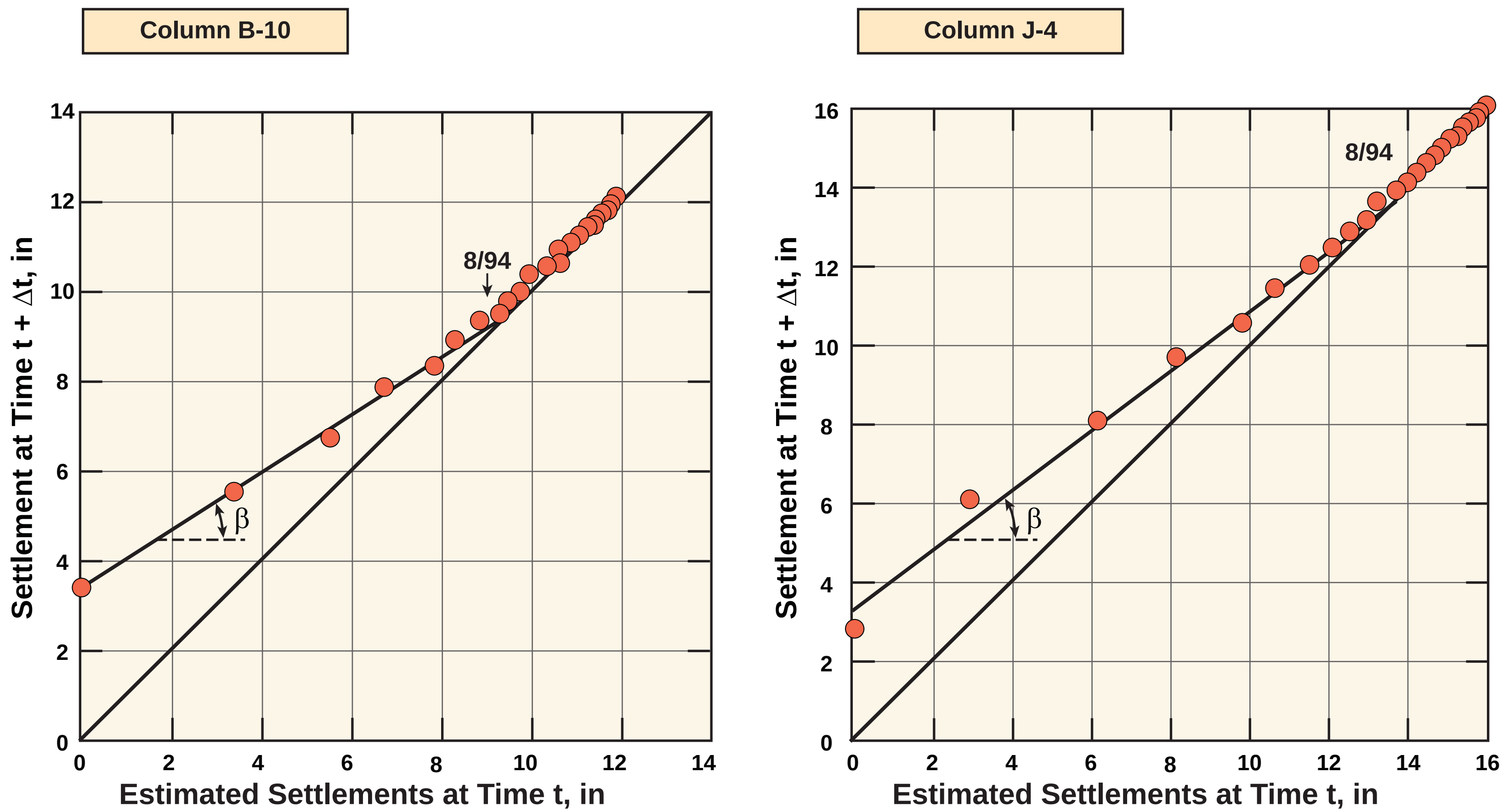



***FIG_65: Asaoka Method for Estimation of Coefficients of Consolidation and End of Primary Consolidation***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_65*

$$c_h = \frac{-D\,e^2\,F(n)\,\ln\beta_1}{8\,\Delta t}; \quad D\,e^2 = 4.4\text{ ft} \quad F(n) = 2.29$$

| Marker ID | $\beta_1$ | $\Delta t$ (yr) | $c_h$ (ft$^2$/yr) | |
|---|---|---|---|---|
| SM-8 | 0.904 | 0.100 | 5.60 | (6.90) |
| | 0.743 | 0.200 | 8.20 | |
| SM-15 | 0.935 | 0.050 | 7.40 | (7.90) |
| | 0.860 | 0.100 | 8.30 | |
| SM-16 | 0.901 | 0.050 | 11.50 | (10.70) |
| | 0.836 | 0.100 | 9.90 | |
| SM-17 | 0.913 | 0.050 | 10.10 | (6.30) |
| | 0.837 | 0.100 | 9.90 | |
| SM-18 | 0.946 | 0.050 | 6.20 | (10.30) |
| | 0.892 | 0.100 | 6.40 | |
| D-21 | 0.740 | 0.110 | 14.75 | |
| D-23 | 0.820 | 0.110 | 9.70 | |
| D-25 | 0.860 | 0.110 | 7.60 | |
| PAD V | 0.800 | 0.140 | 9.00 | |
| Post-Preload Measurements 12 Columns | 0.743 | 0.273 | 6.20 | |
| Average During Preload Period | | | 8.80 | |

*FIG_66: Estimates of coefficients of Horizontal Consolidation Using the Asaoka Method (Jamialkowski, et al., 1995)*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\FIG_66*

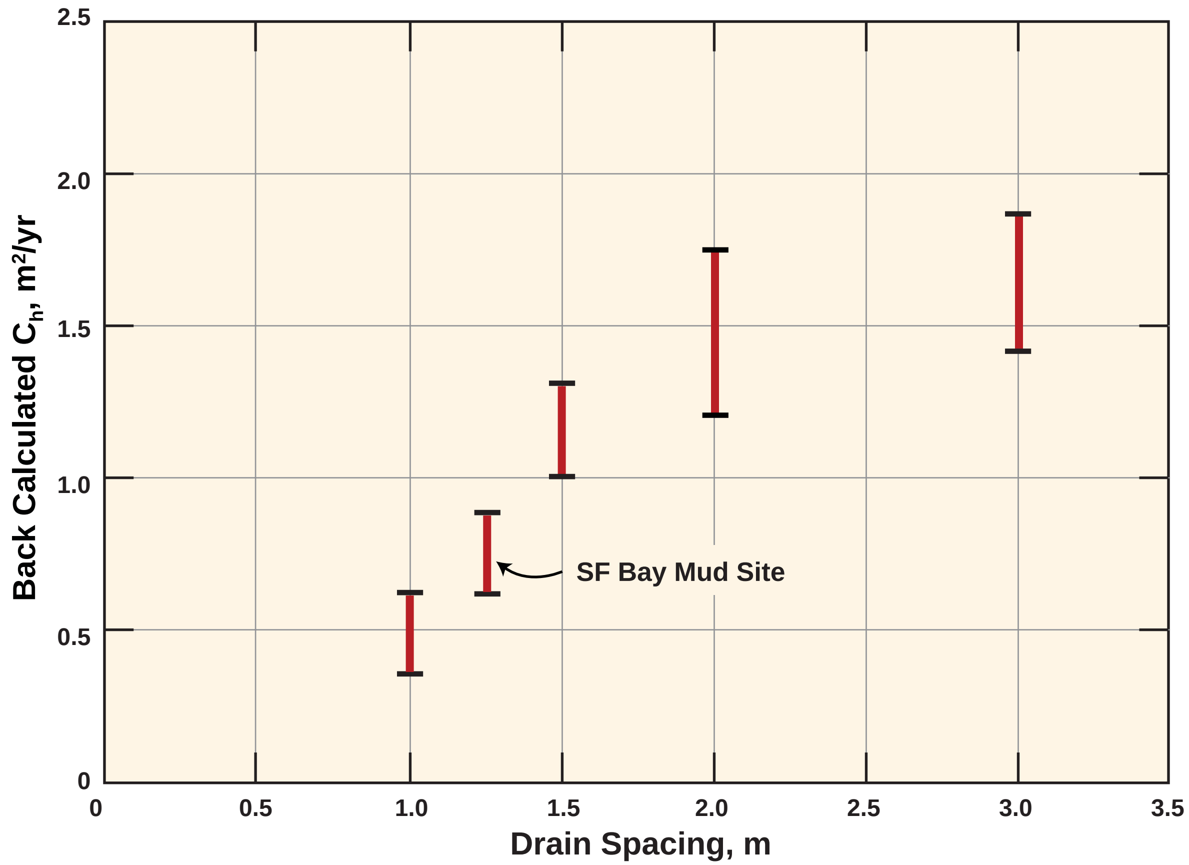



***FIG_67: Variation of Ch with Drain Spacing***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_00*

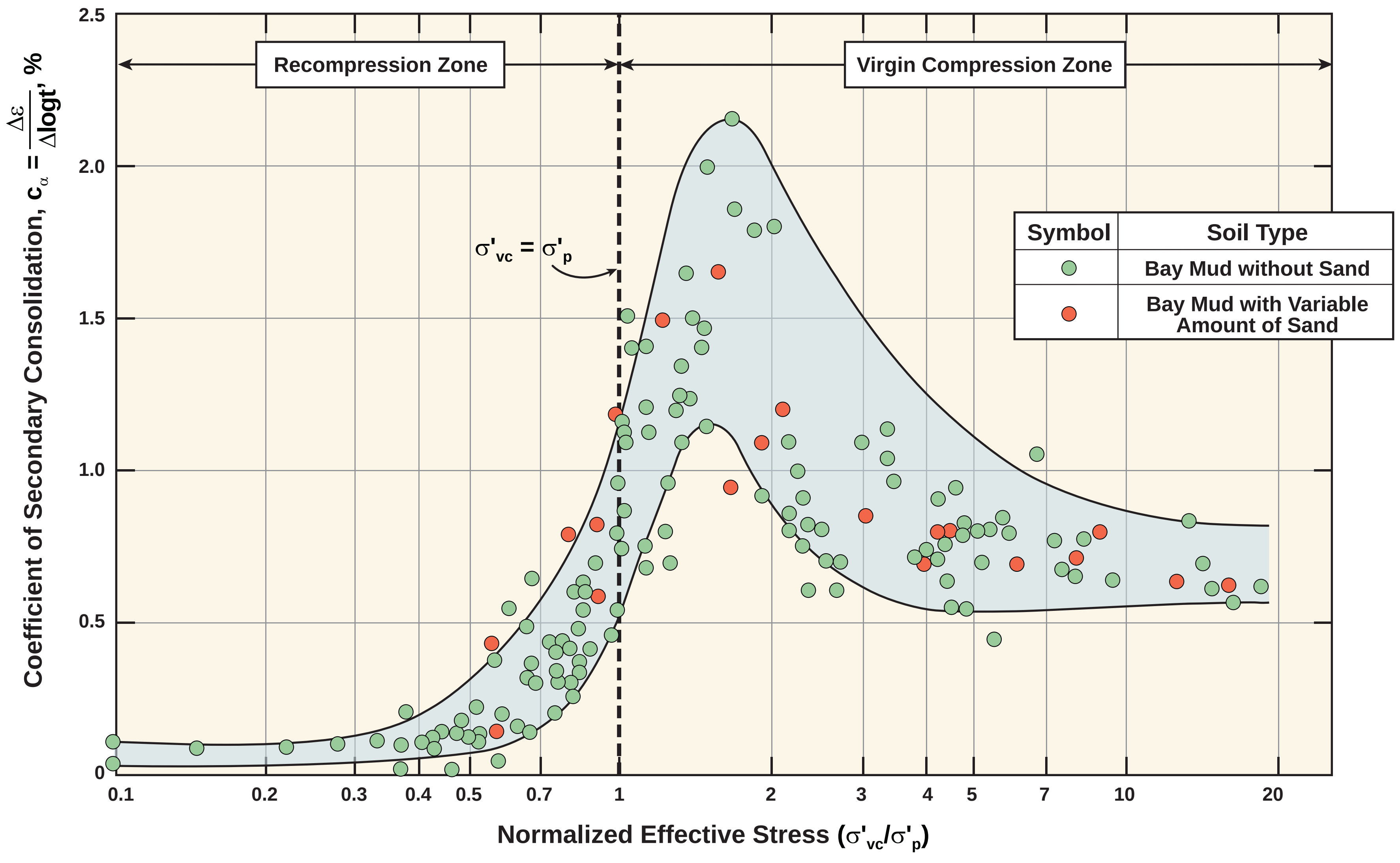



***FIG_68: Variation of Coefficients of Secondary Consolidation with Consolidation Stress***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\FIG_68*

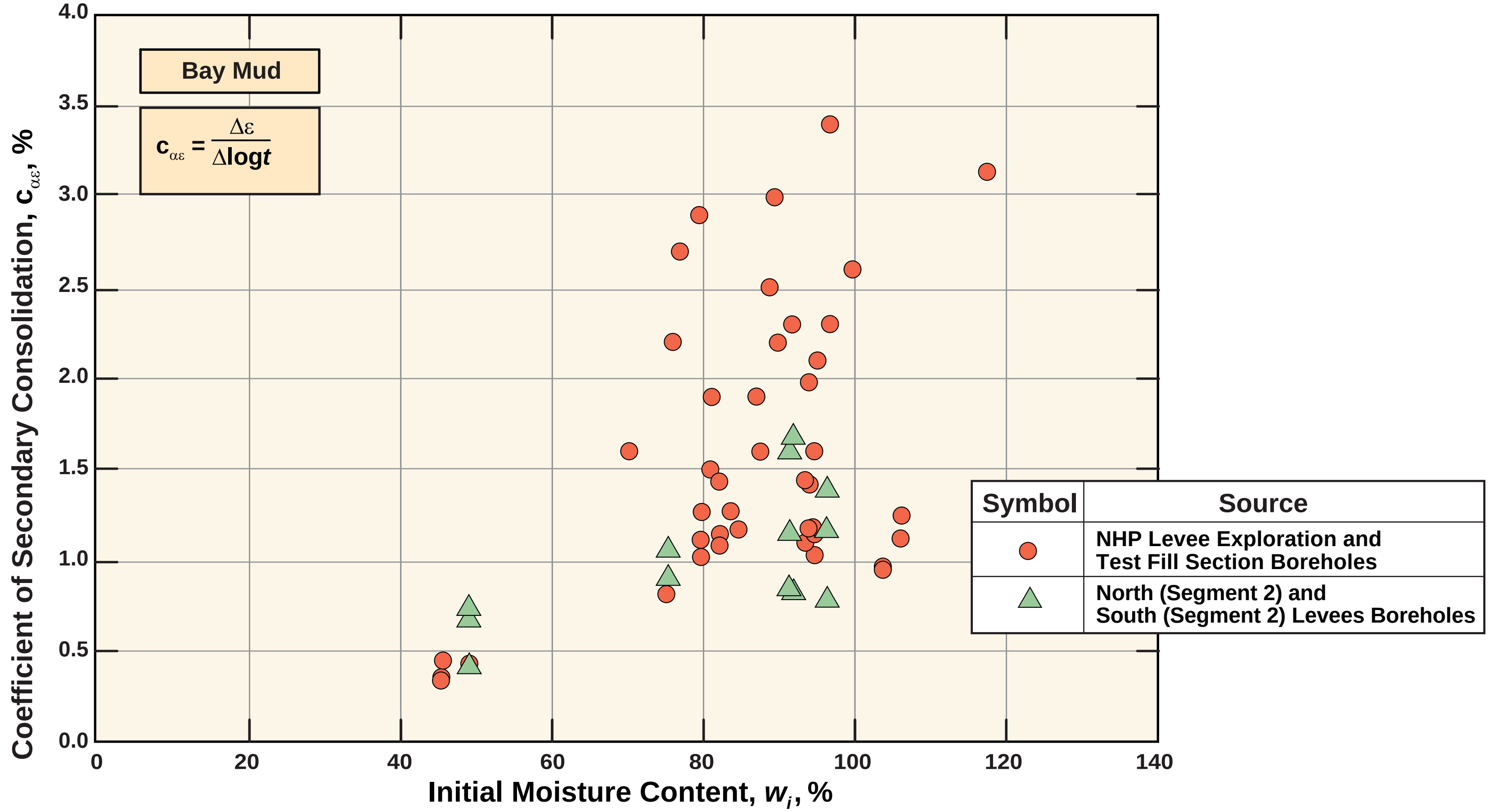



***FIG_69: Variation of Coefficient of Secondary Consolidation with Moisture Content***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_69*

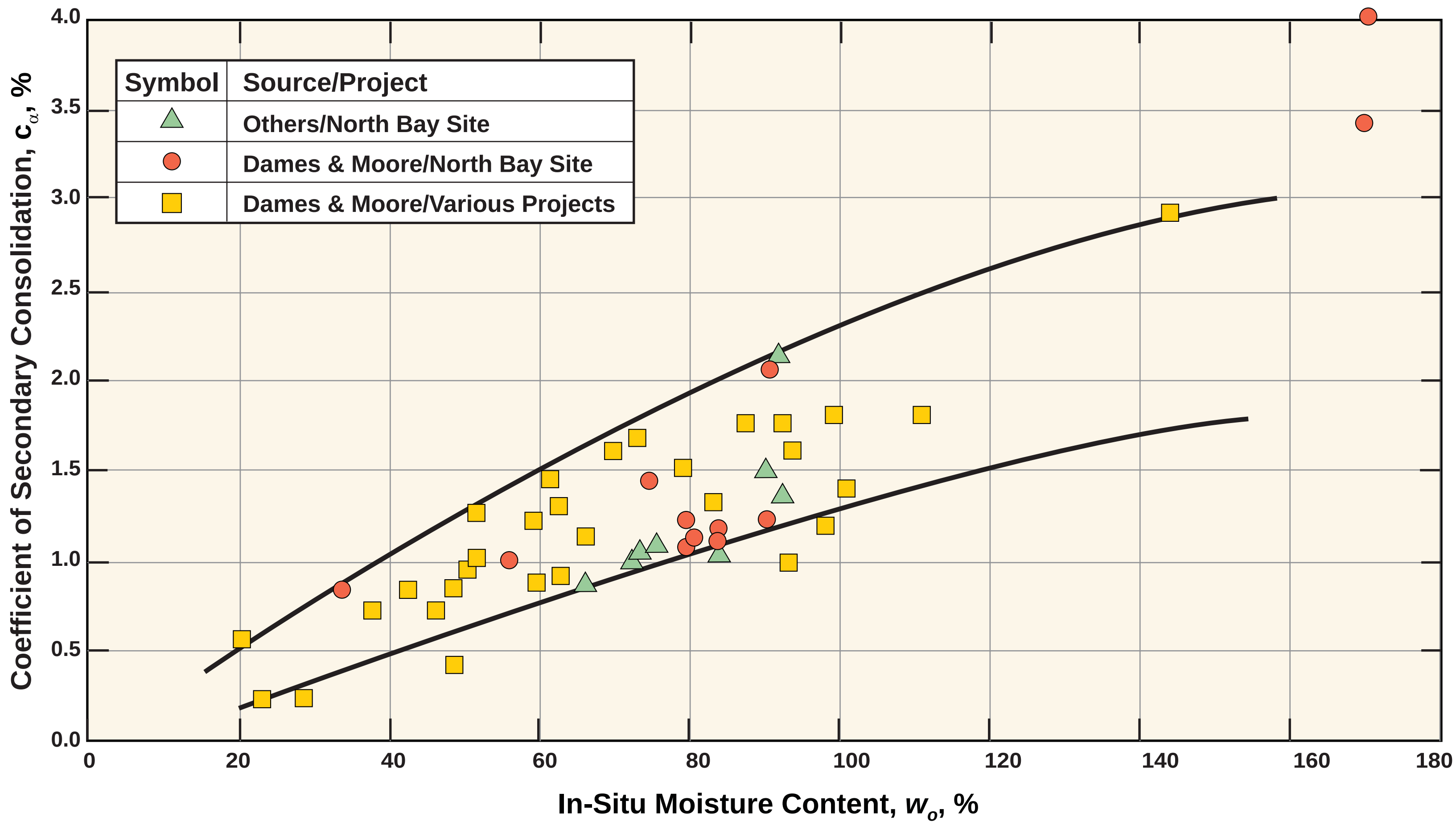



***FIG_70: Variation of Cα with Moisture Content***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\03 BAY MUD (18-78)\FIG_70*

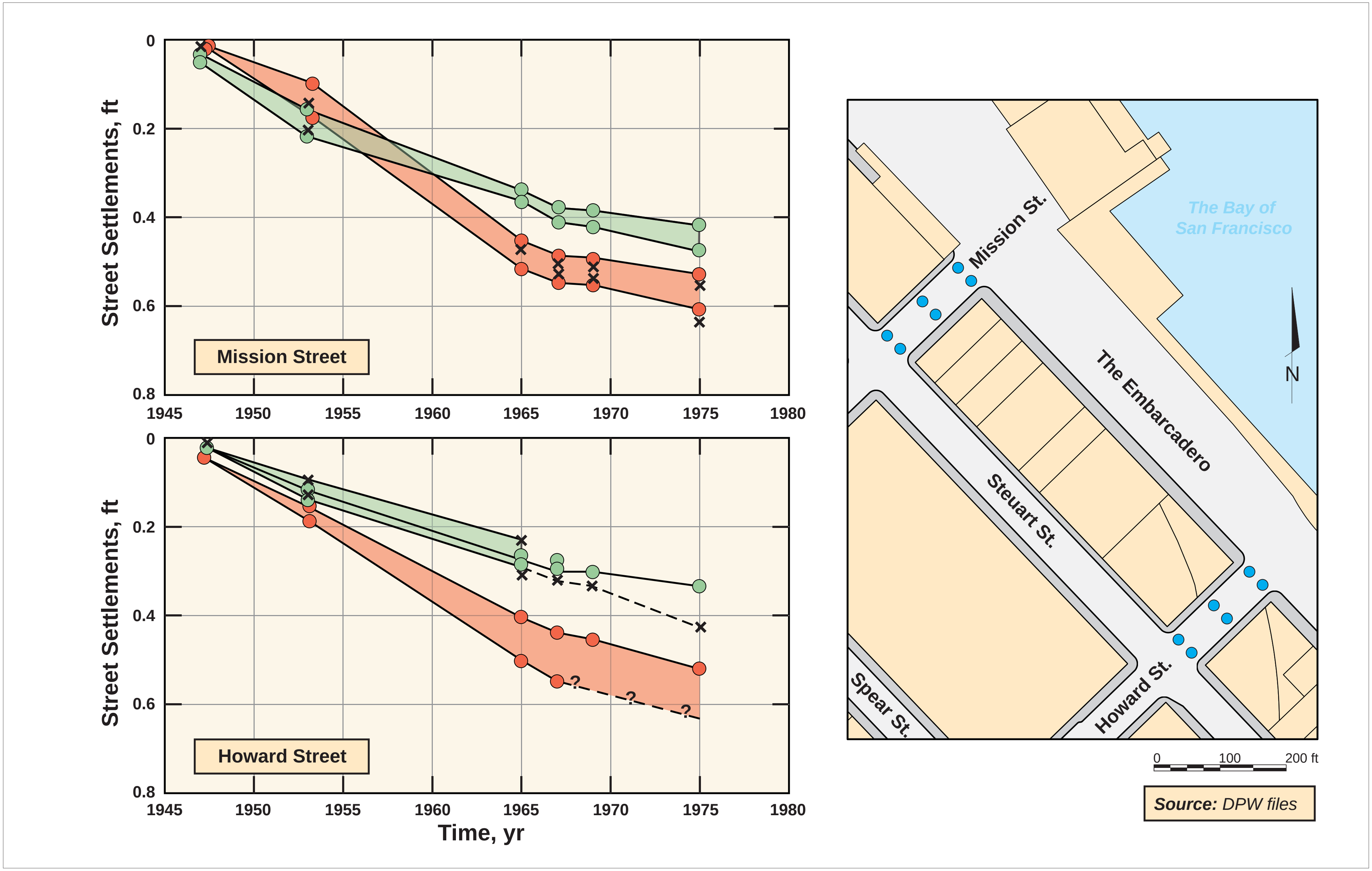



***FIG_71: Long-Term Settlements at Mission and Howard Streets***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\FIG_71*

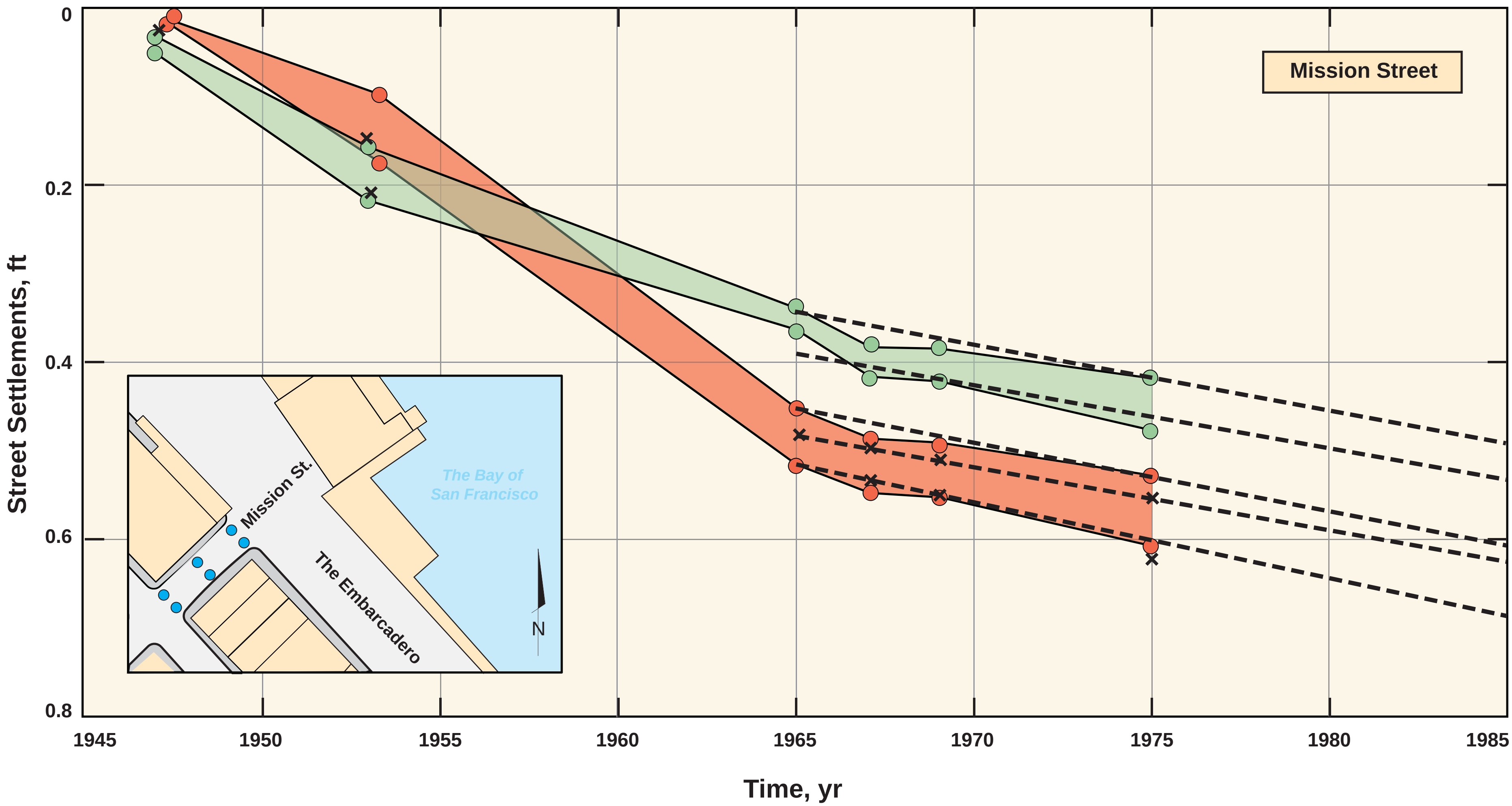



***FIG_72: Extrapolation of Settlement Data at Mission Street For Secondary Compression***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\FIG_72*

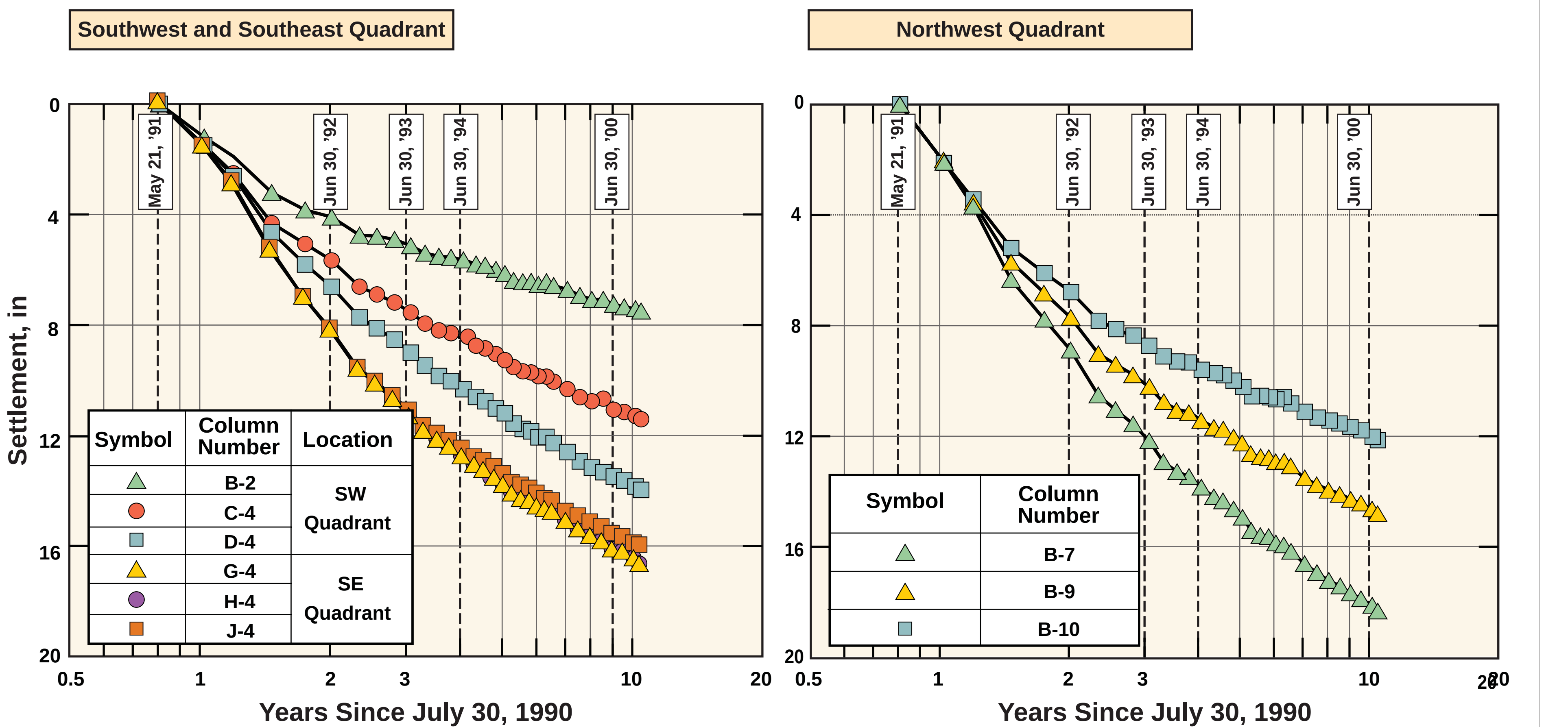
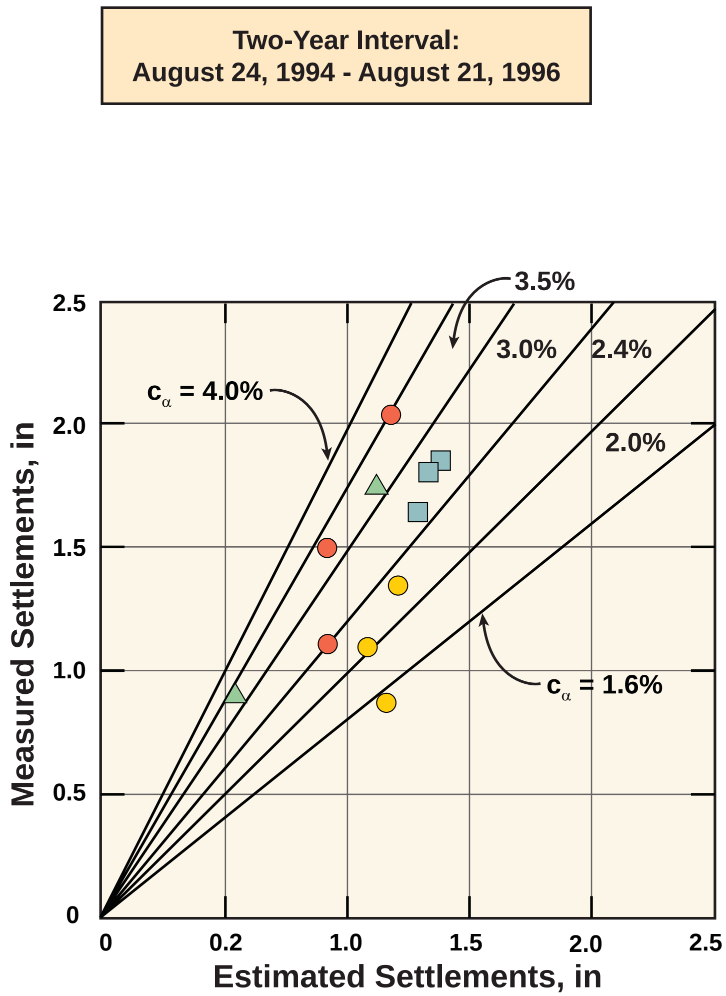



*FIG_73: Estimation of Coefficients of Secondary Consolidation from Long-Term Settlement Data*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_73*



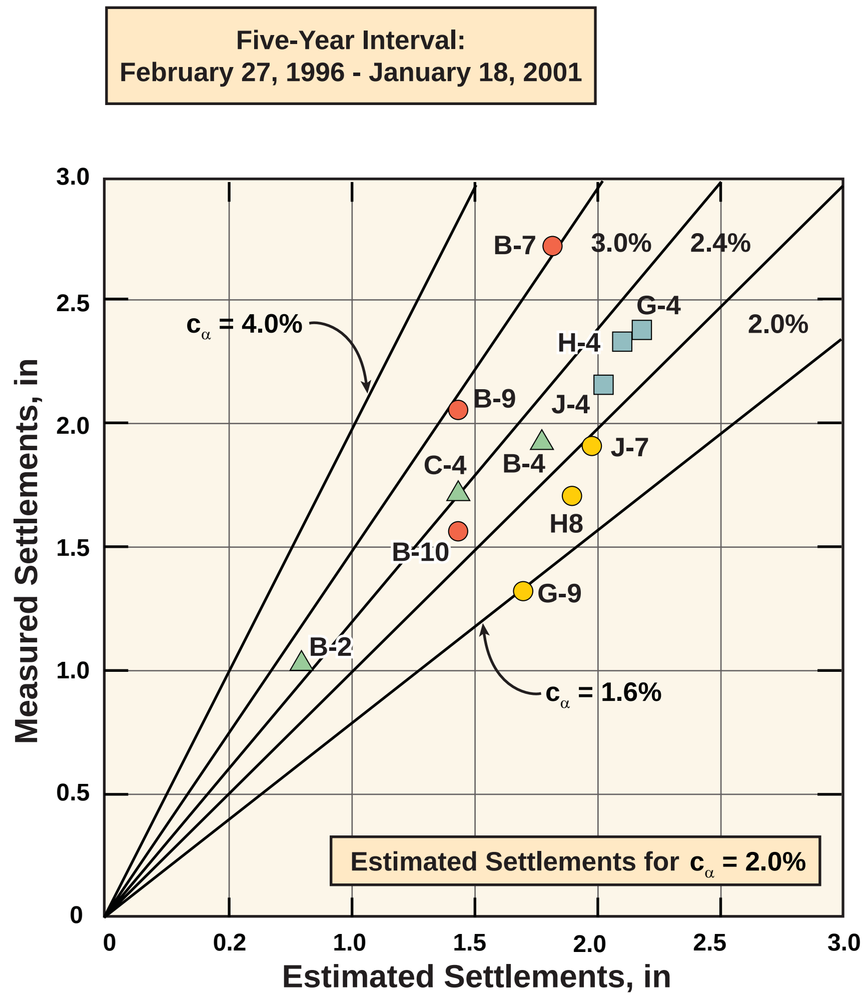
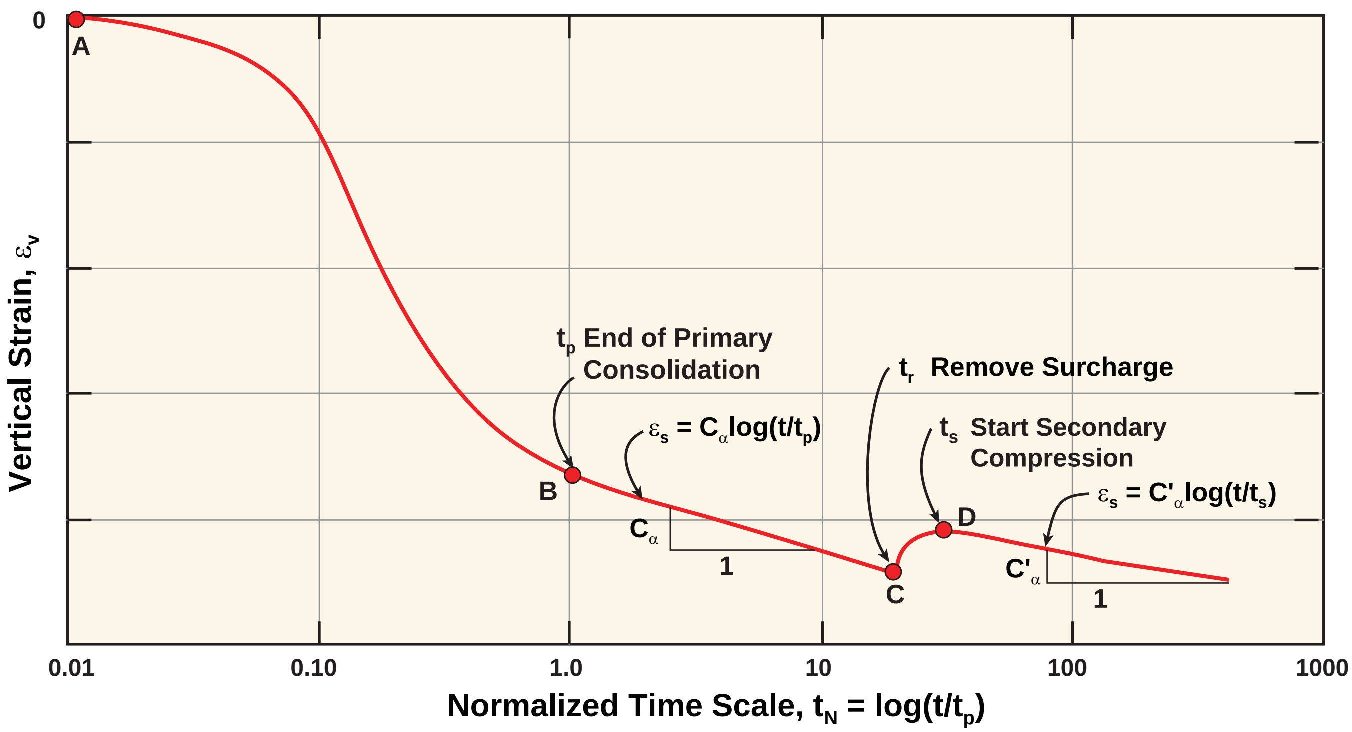
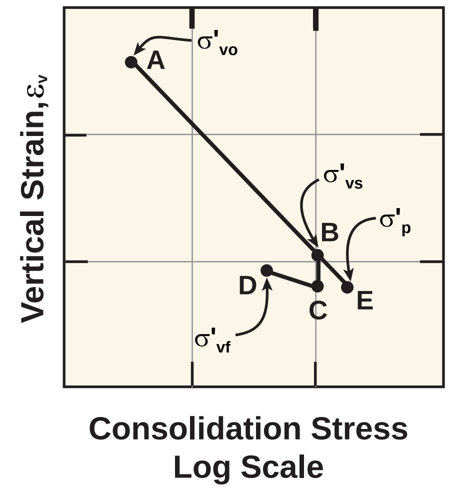




***FIG_74: Estimation of Coefficients of Secondary Consolidation from Settlement Data North Bay Site***

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_74*

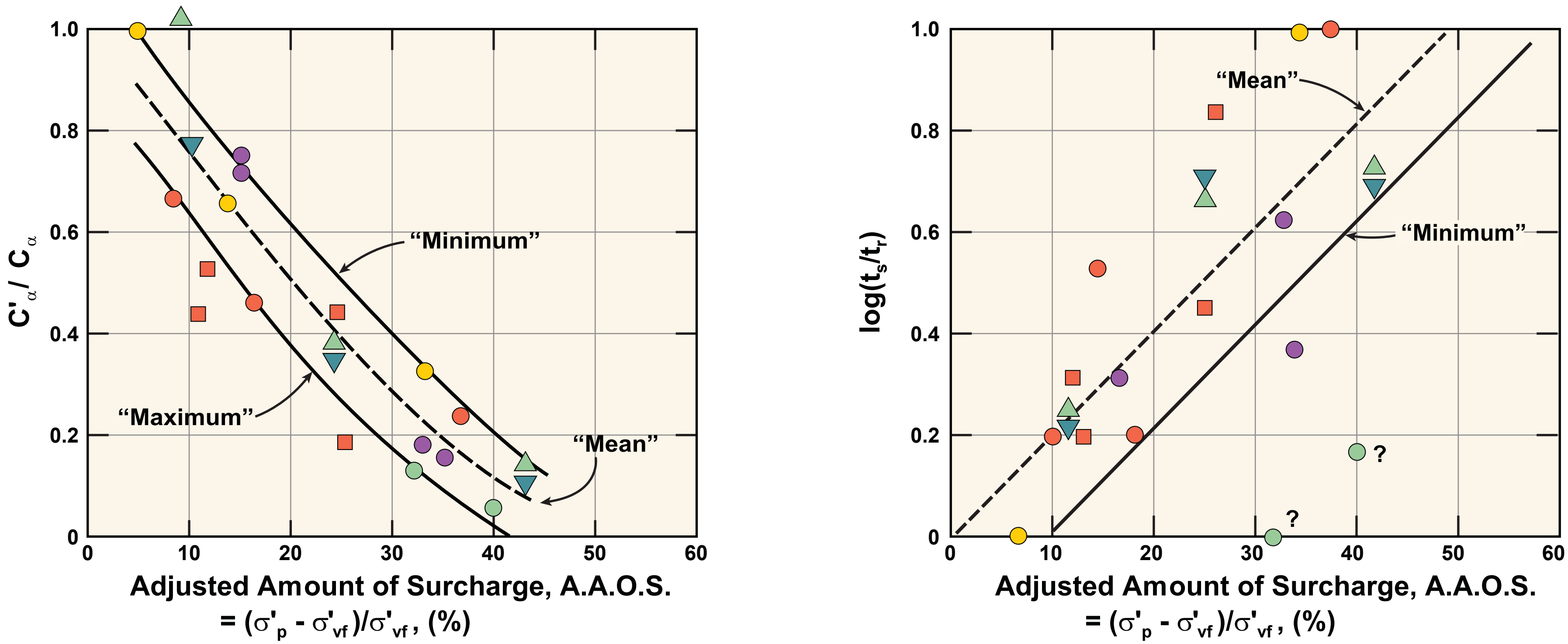



| | |
|---|---|
| $\text{A.O.S.} = \dfrac{\sigma'_{vs} - \sigma'_{vf}}{\sigma'_{vf}}$ | Amount of Surcharge |
| $\text{A.A.O.S.} = \dfrac{\sigma'_{p} - \sigma'_{vf}}{\sigma'_{vf}}$ | Adjusted Amount of Surcharge |

*FIG_75: Effects of Surcharge on Secondary Compression (Ladd, 1989)*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Plates\FIG_75*




| Reference (Location) | Soil | $\frac{c_\alpha}{CR}$ | $\frac{t_r}{t_p}$ | Symbol |
|---|---|---|---|---|
| Koutsoftas et al. (1987) (Hong Kong) | Upper Marine CH $I_p$ ~ 55% | 0.027 ± 0.001 | ≤ 1.0 | ▲ ▼ |
| Simons (1965) (Fornebu, Norway) | Marine CH $I_p$ ~ 30% | Est. 0.04 | 1.0 2.0 | ● ● |
| Haley & Aldrich (1987) (Hartford, Conn.) | Varved Clay $w_o$ = 64 ± 5% | 0.02- 0.025 | 1.0 - 1.8 | ■ |
| GZA (1988) (Sommerville, Ma.) | Organic Silt $w_l$ = 95%, $I_p$ ~ 54% | 0.046 ± 0.003 | 3.5 ± 0.5 | ● |
| MRCE (1989) (Syracuse, N.Y.) | Varved Clay $w_o$ = 46% | 0.041 ± 0.003 | 21 ± 5 | ● |

*FIG_76: Reduction of Coefficient of Secondary Consolidation Due to Surcharge (Ladd, 1989)*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_76*

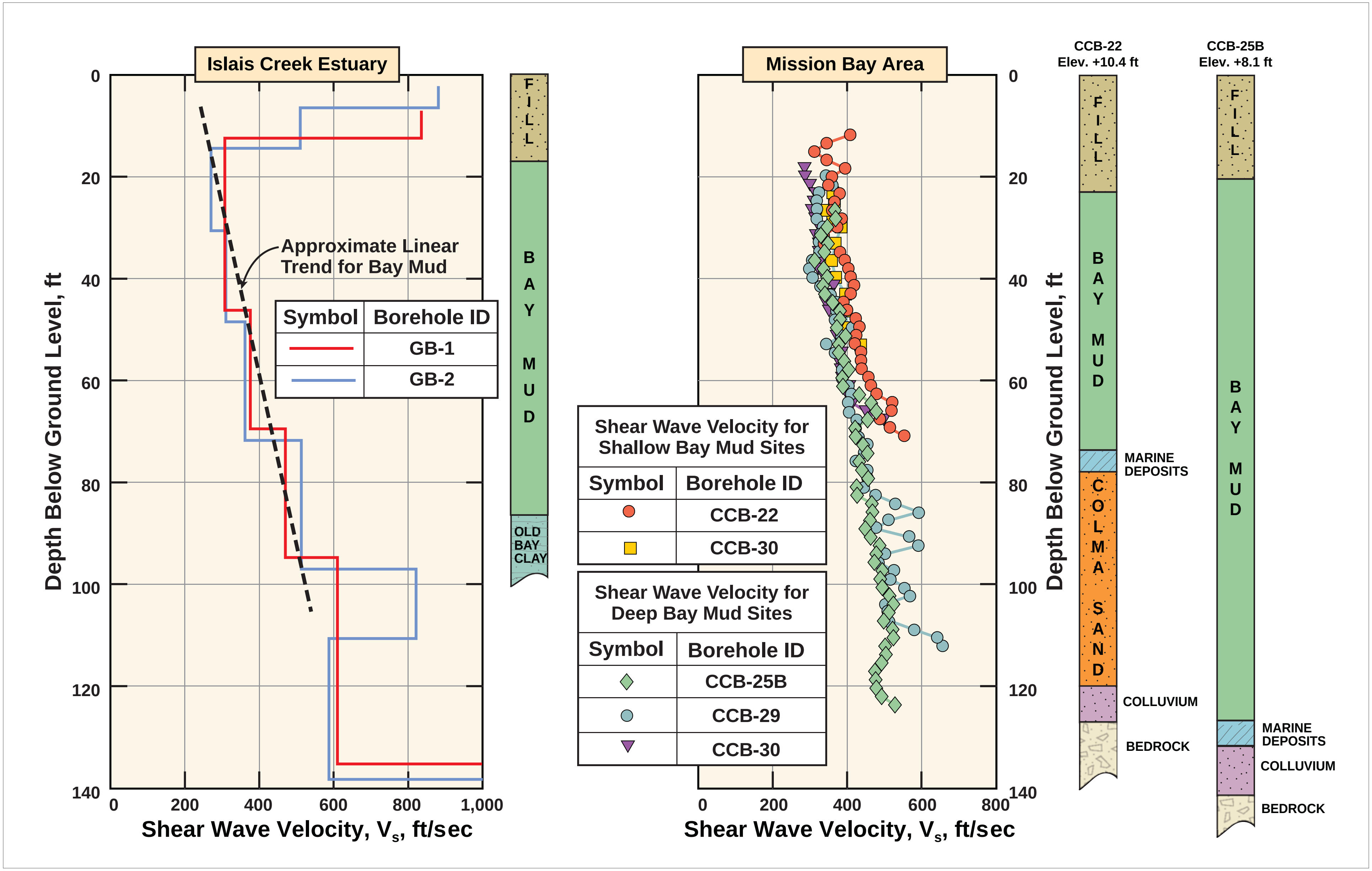



*FIG_77: Shear Wave Velocities of Bay Mud*

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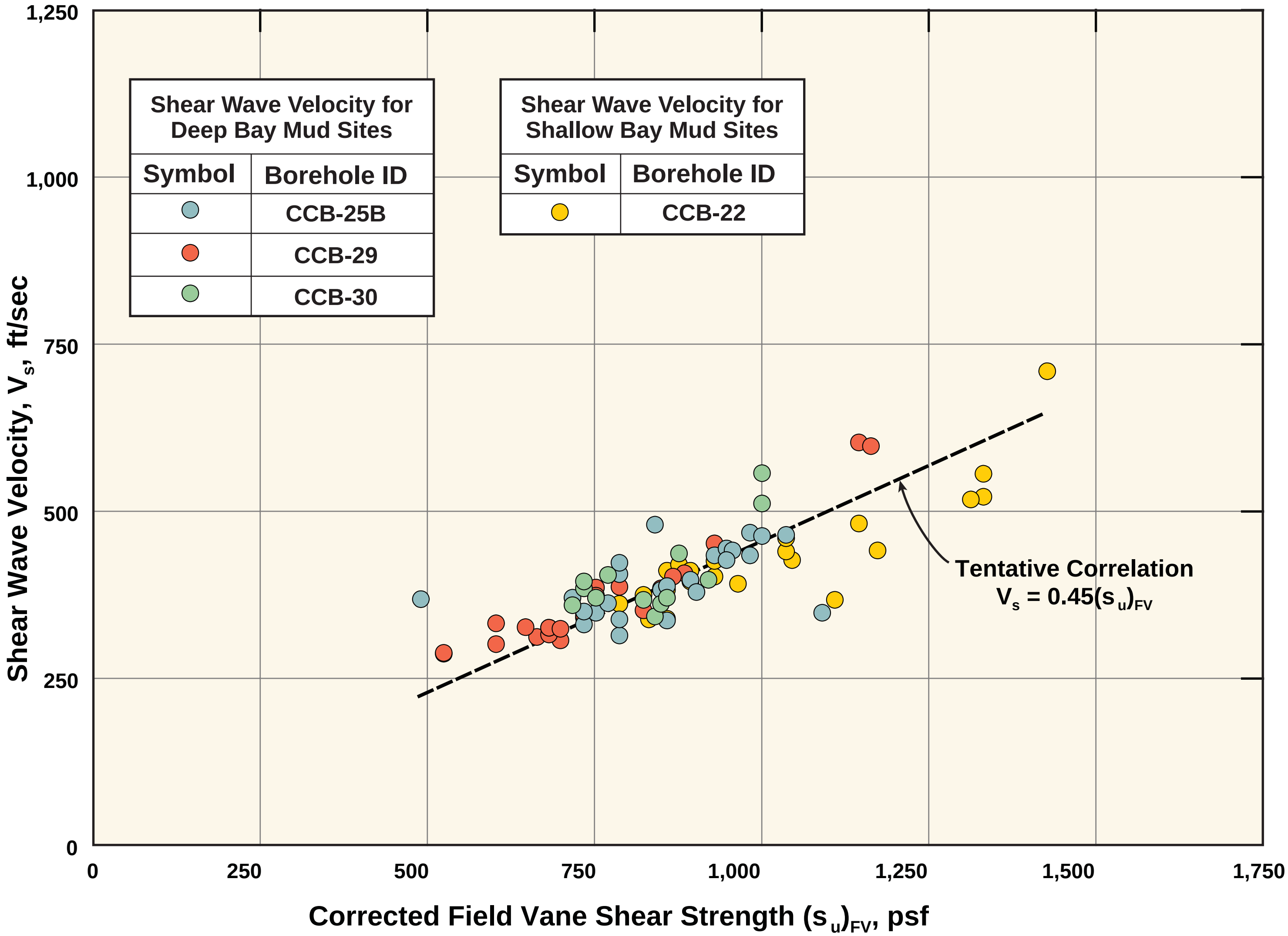


*FIG_78: Tentative Correlation of Shear Wave Velocity with Undrained Shear Strength of Bay Mud*

*W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\03 BAY MUD (18-78)\FIG_78*