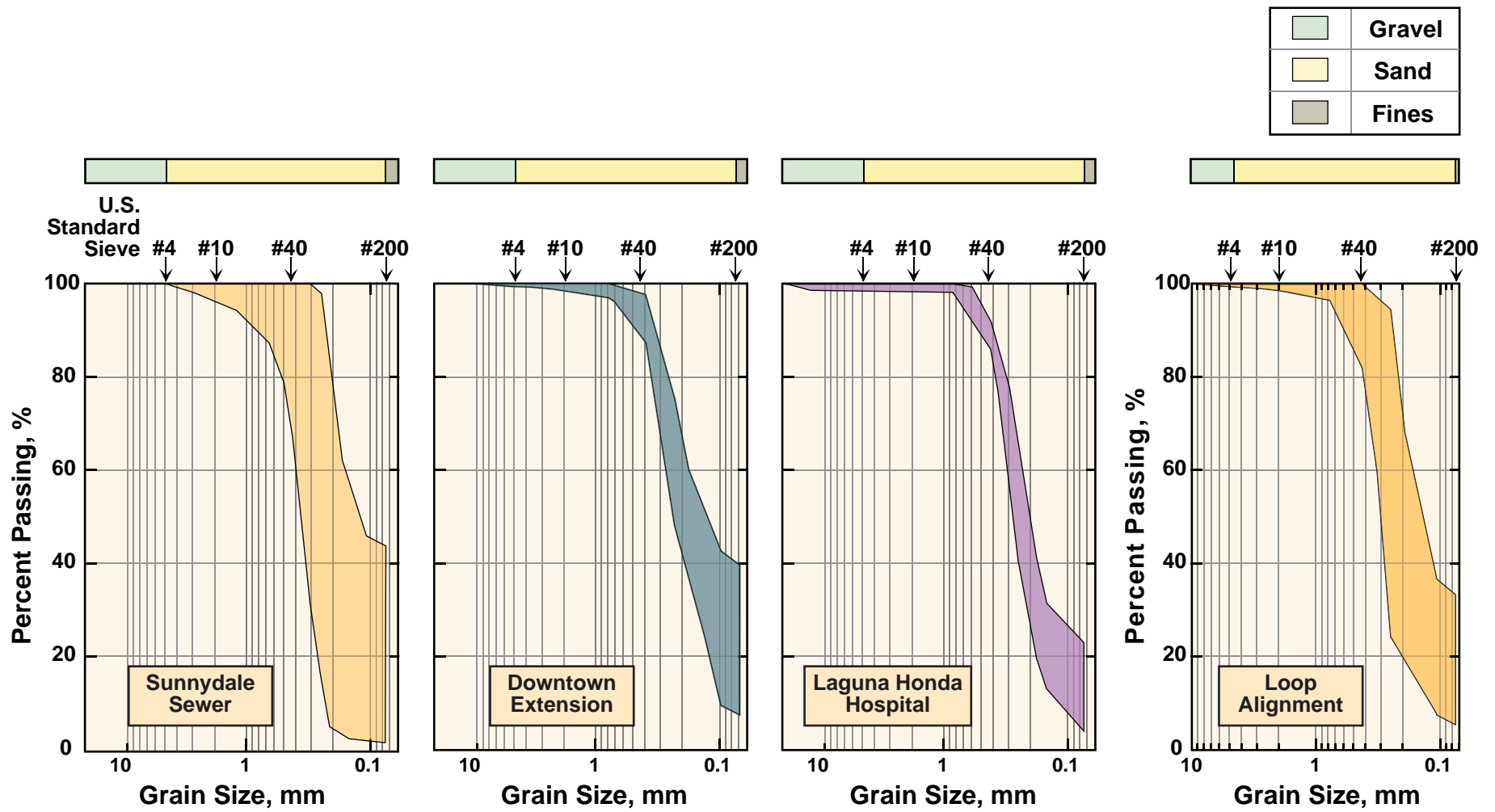
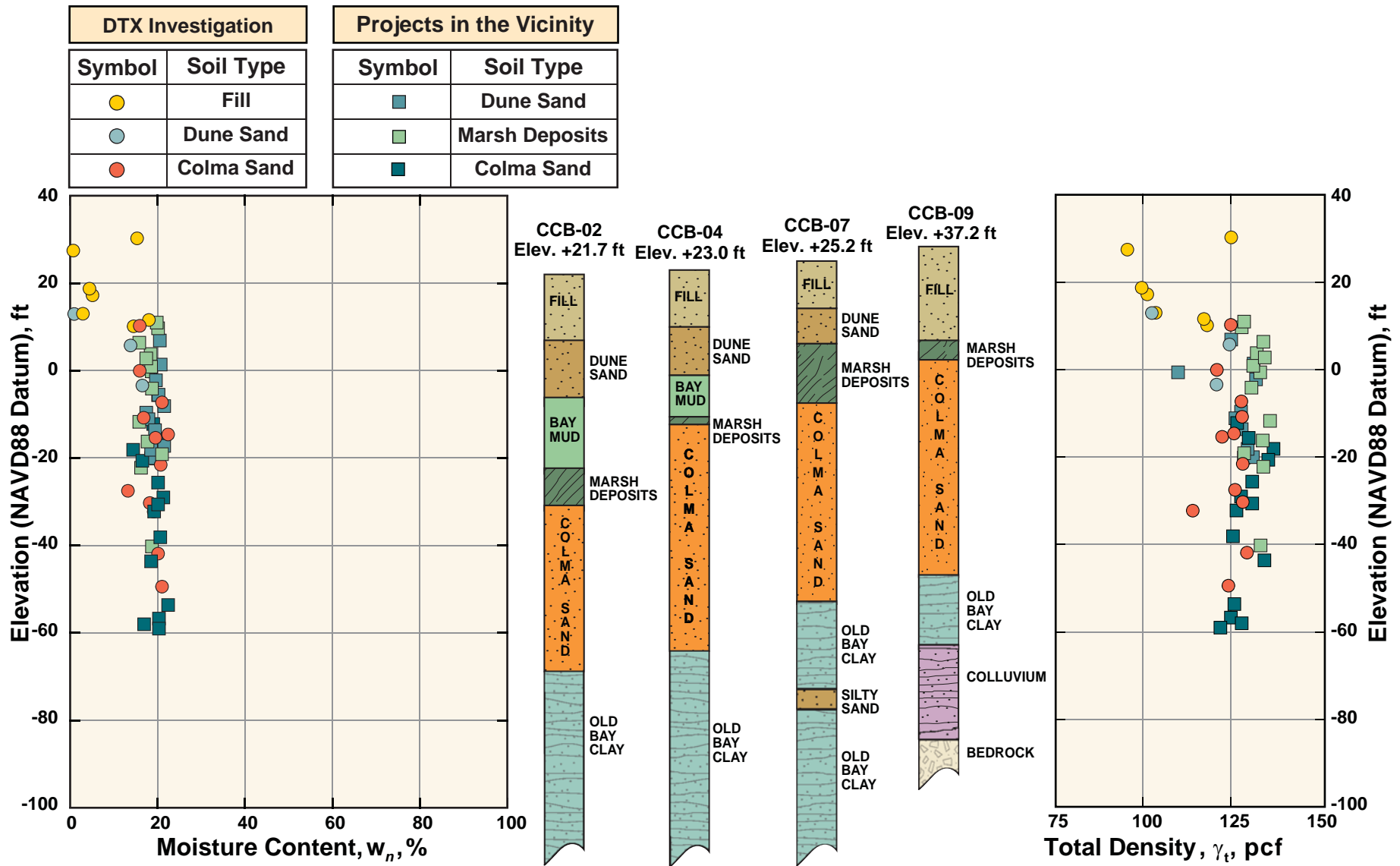


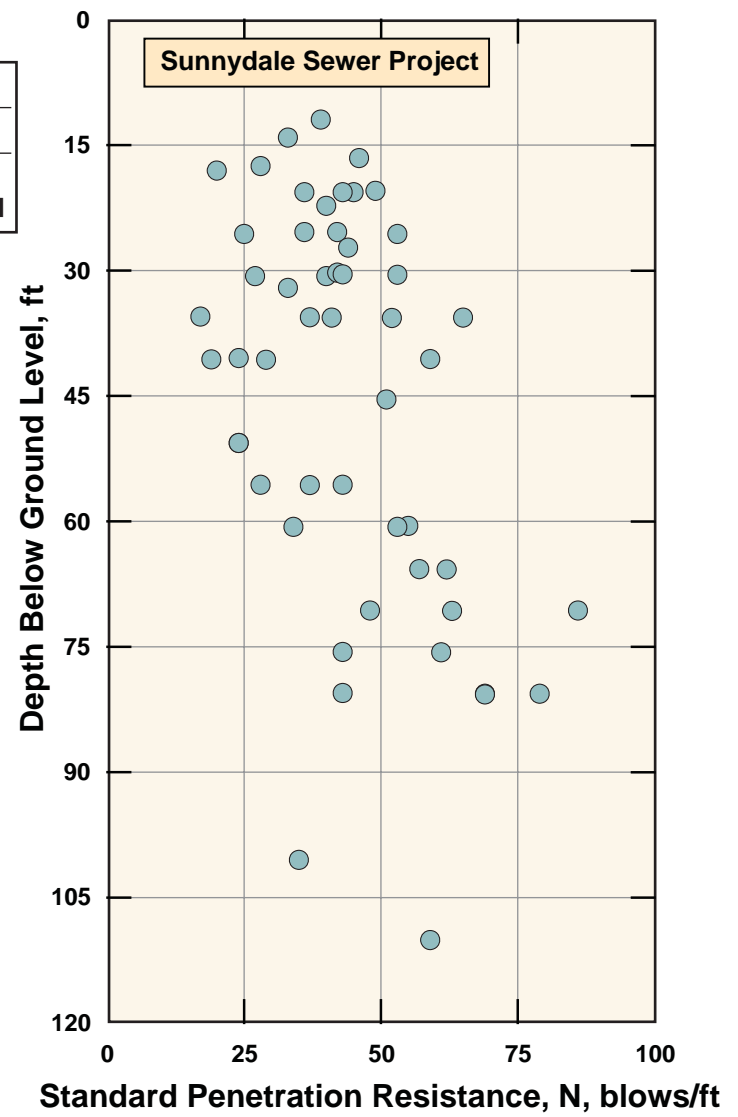
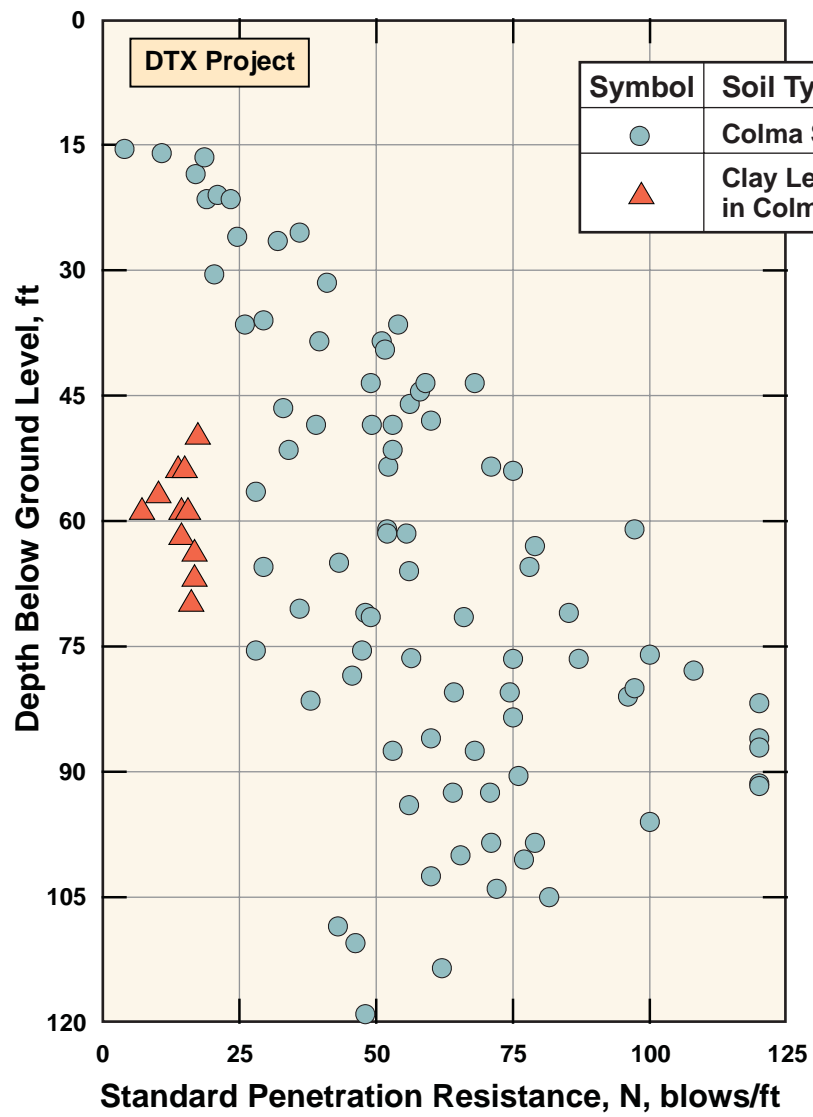
Colma Sand



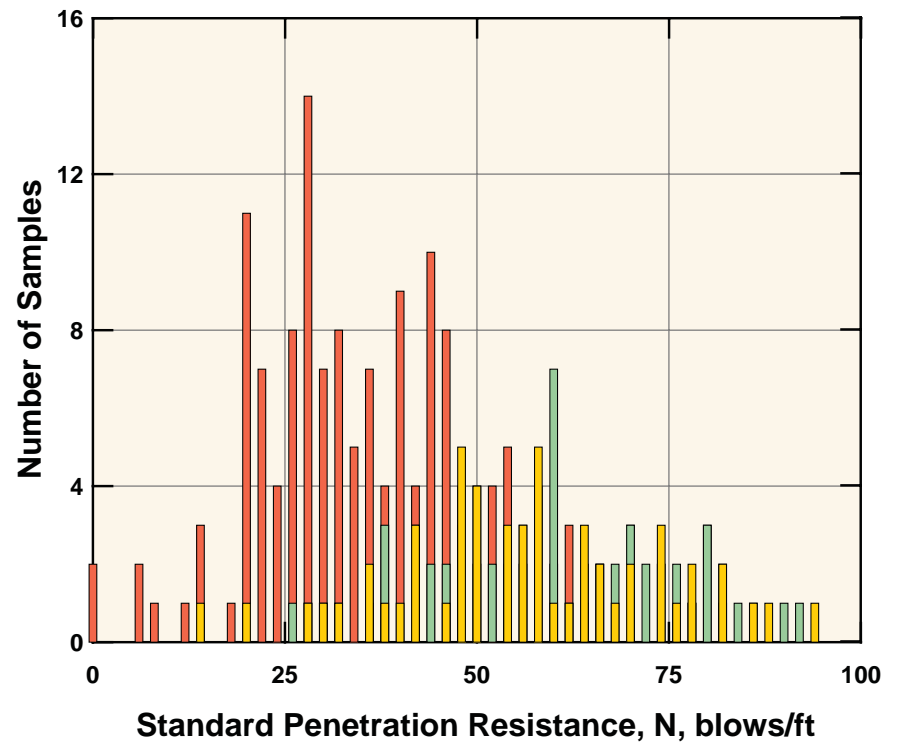
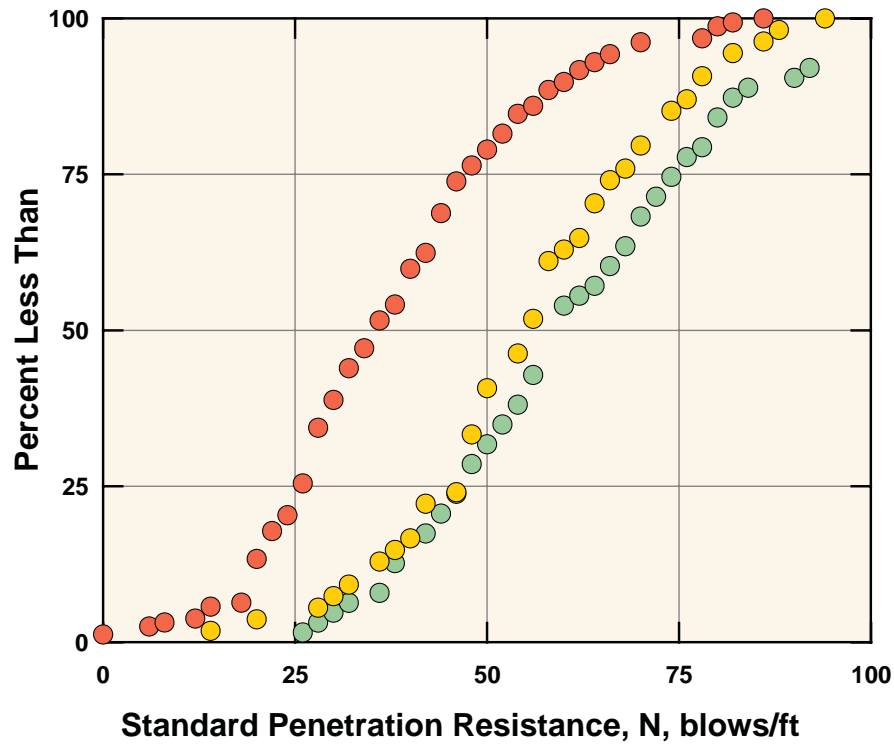
FIG_79A: Typical Gradations of Colma Sand from Four Sites



FIG_80: Moisture Contents and Total Densities of Colma Sand

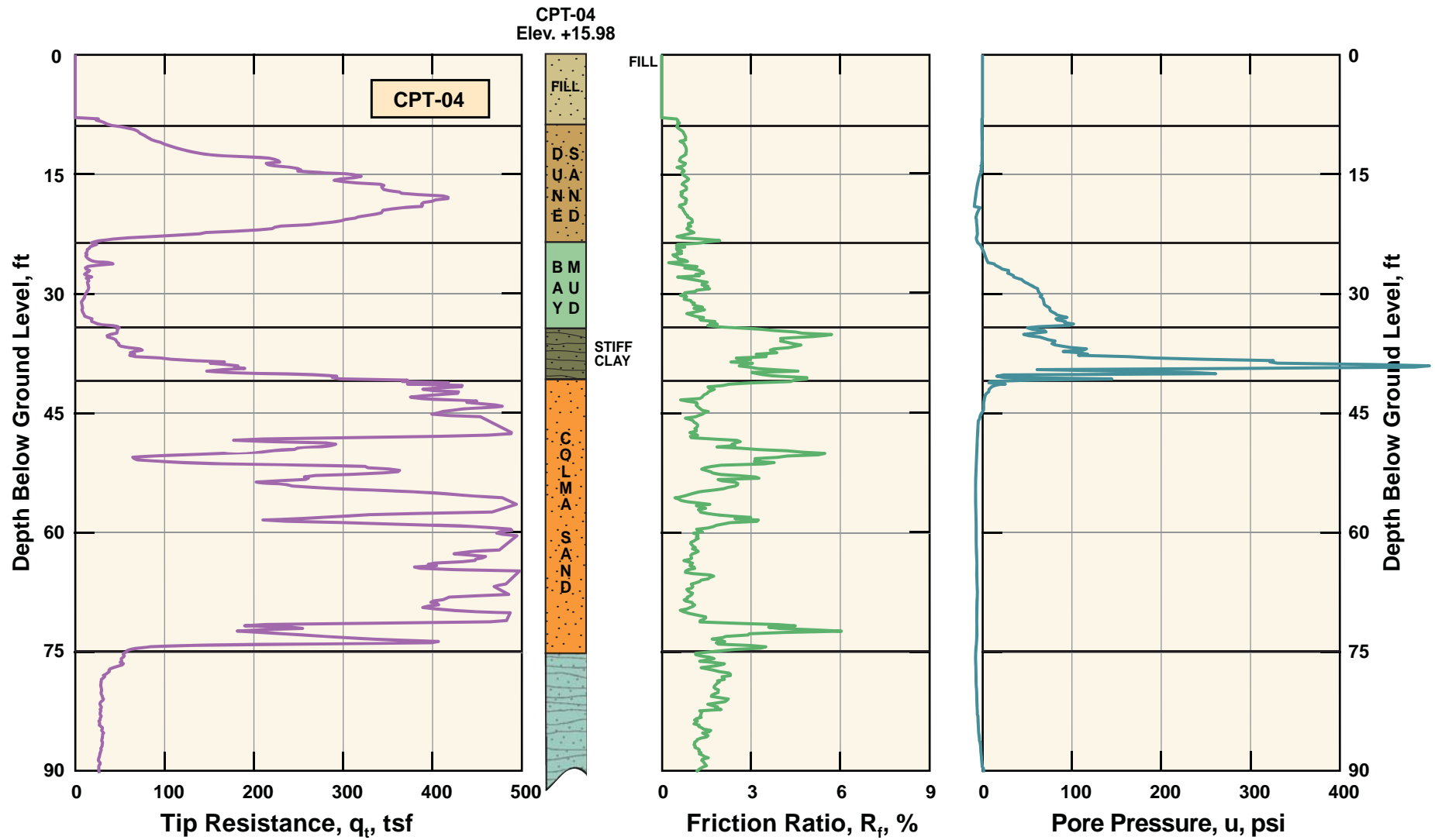


FIG_81: Variation of SPT N Values of Colma Sand



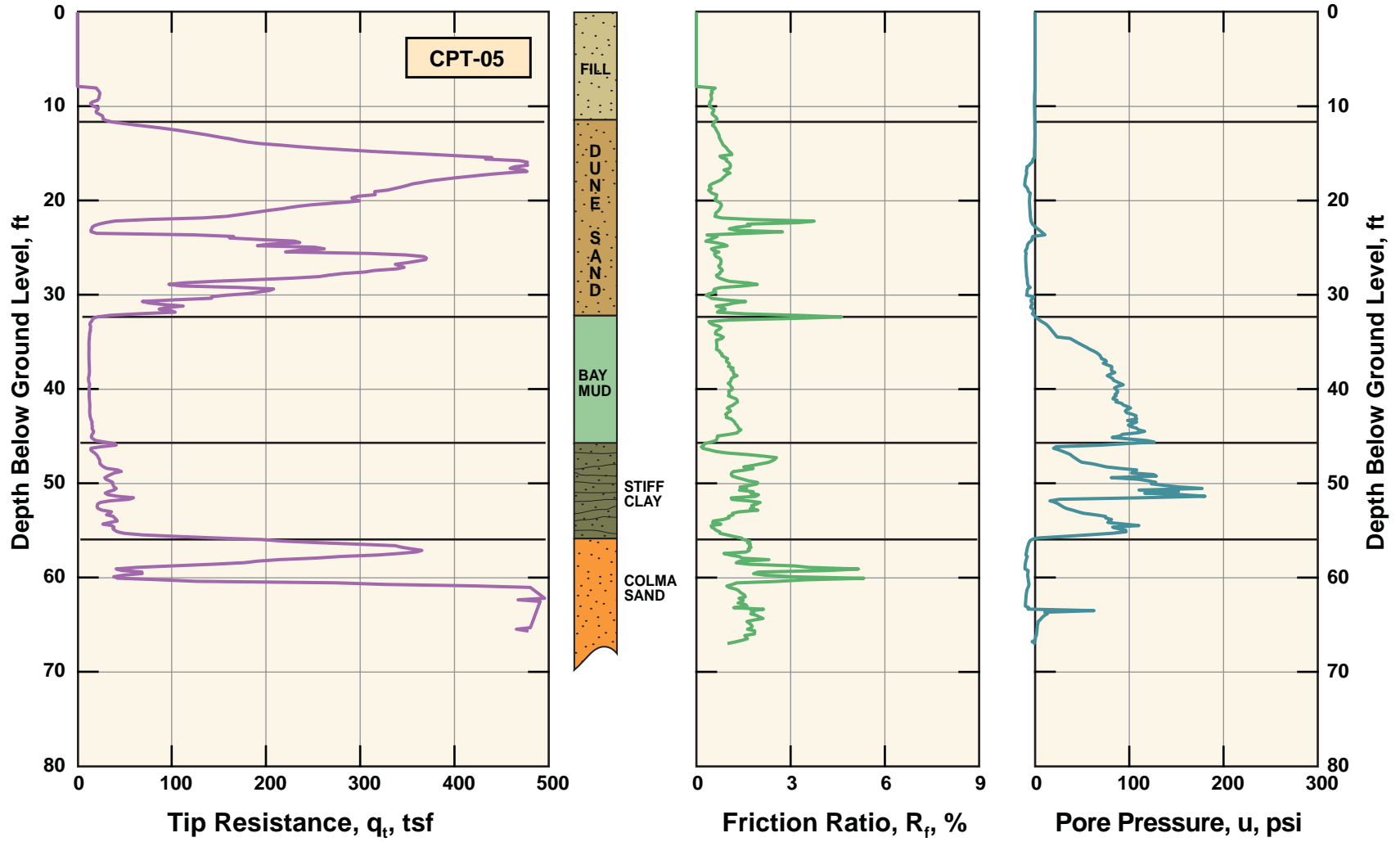
Project	Downtown Extension	Sunnydale Sewer	Laguna Honda
Symbol	●	●	●
Number of Tests, N	63	157	54
Mean, μ , blows/ft	63	38	55
Standard Deviation, σ , blows/ft	23	17	17

FIG_82: Statistical Analyses of SPT N Values of Colma Sand from Three Sites

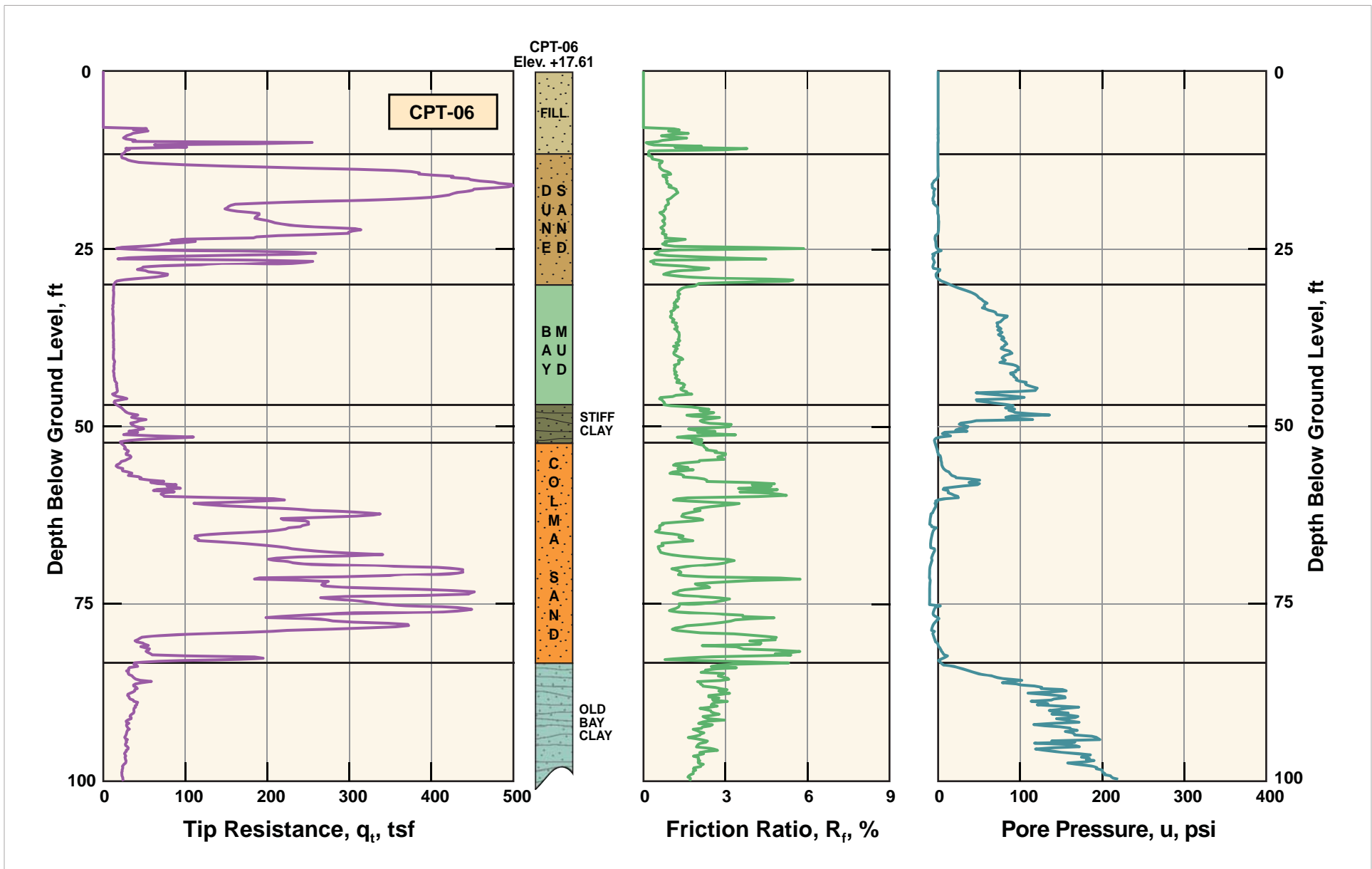


FIG_83: CPT Test in Colma Sand

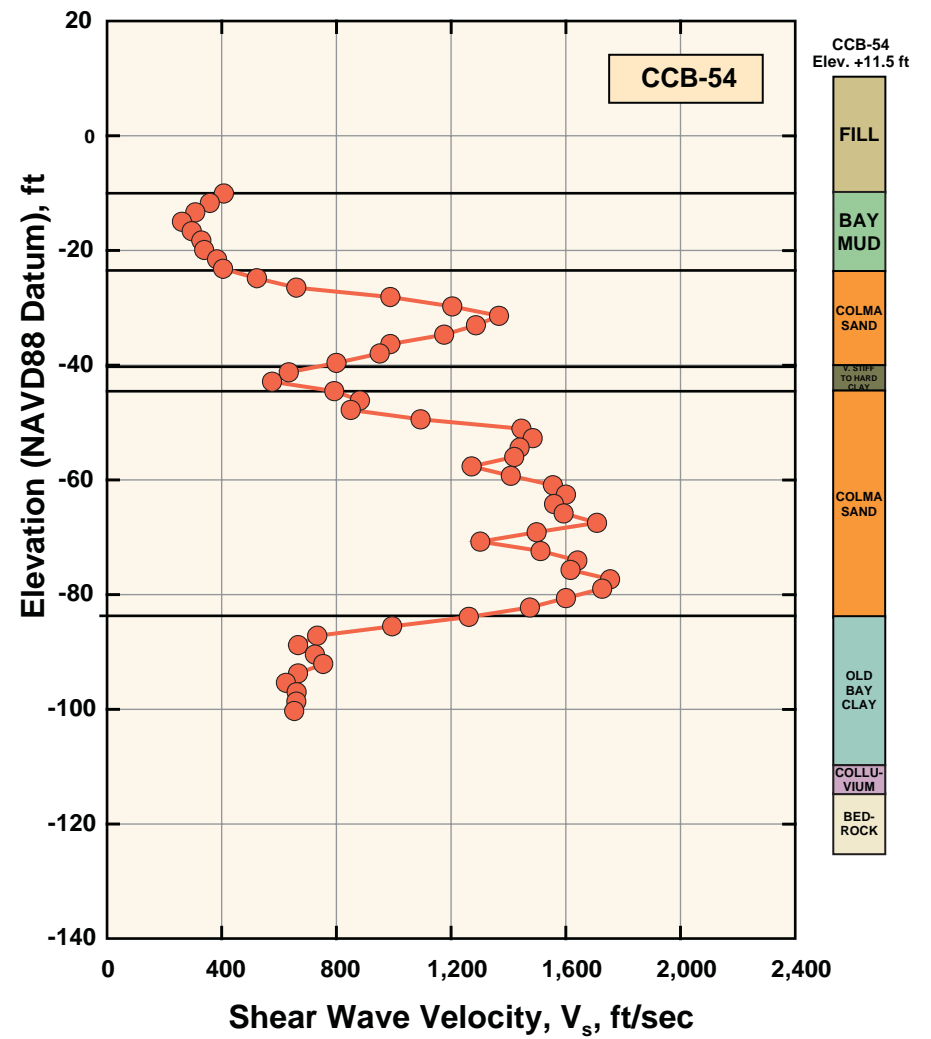
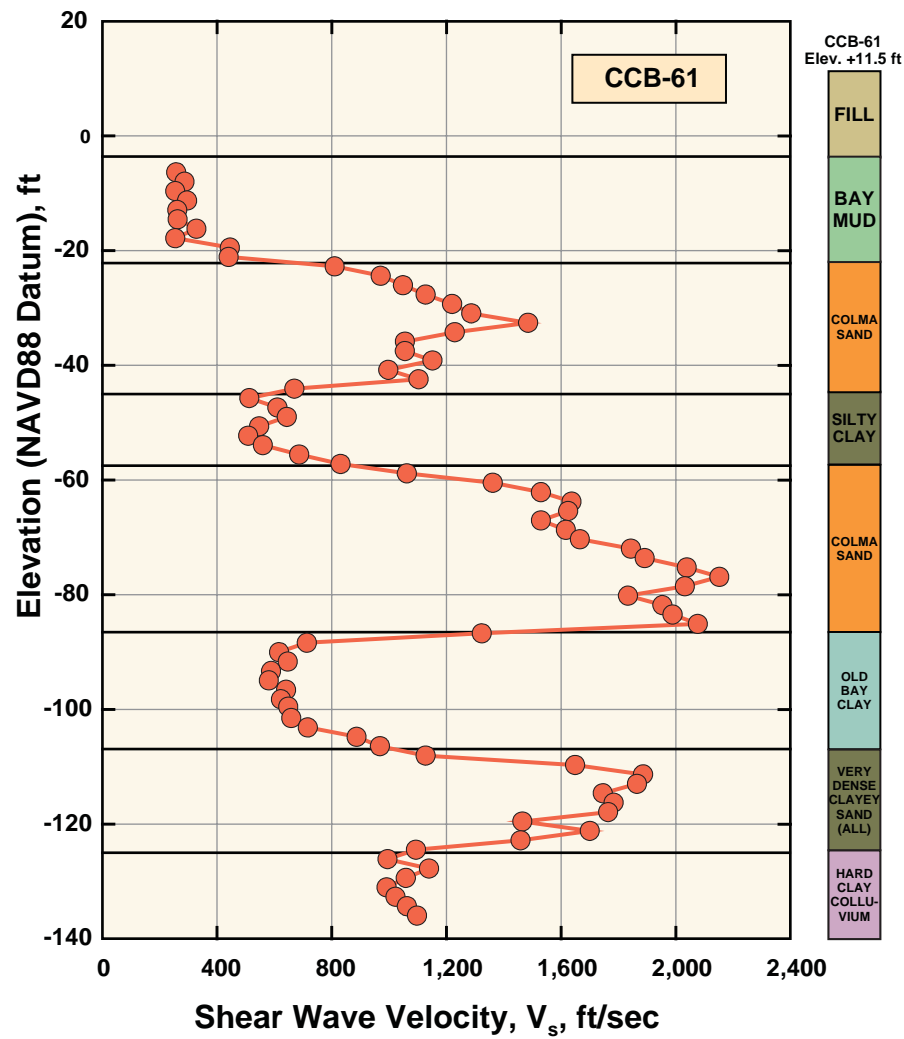
CPT-05
Elev. +18.09



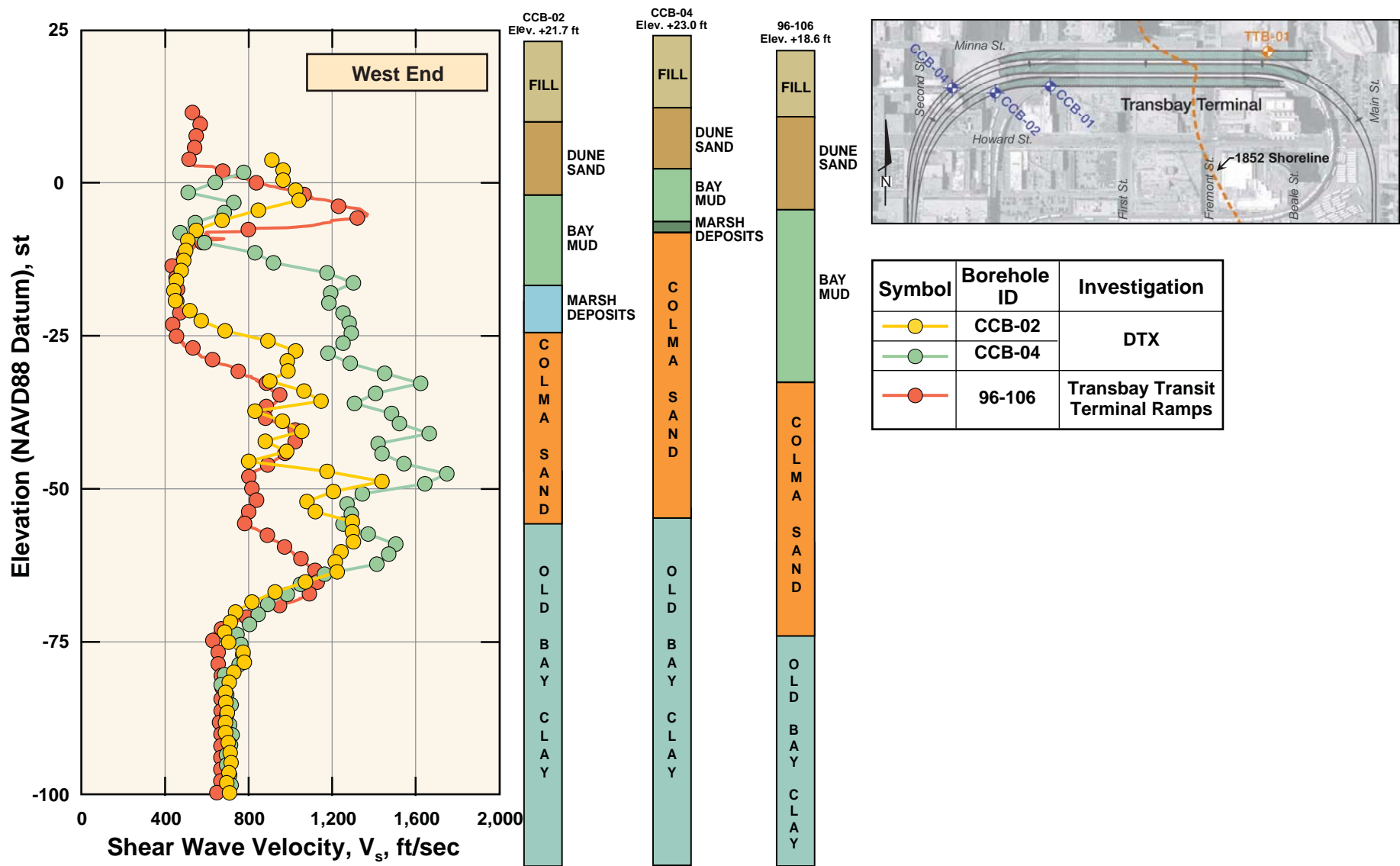
FIG_84: Refusal of CPT Tests in Colma Sand



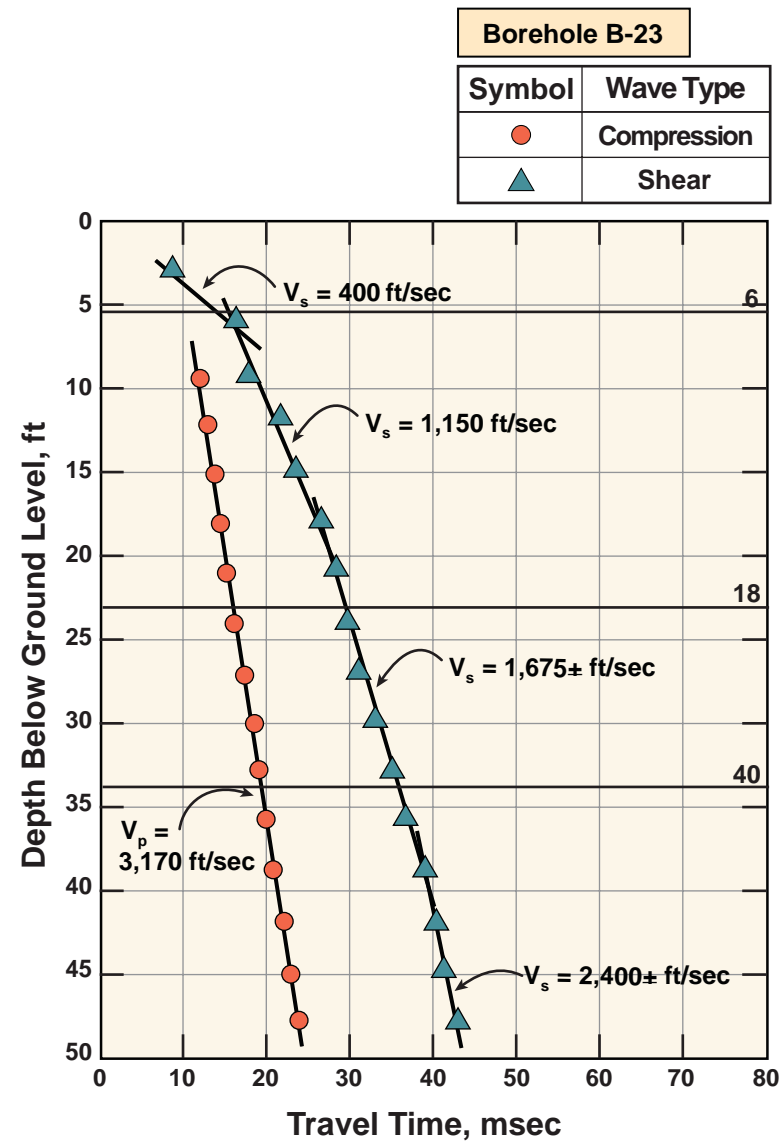
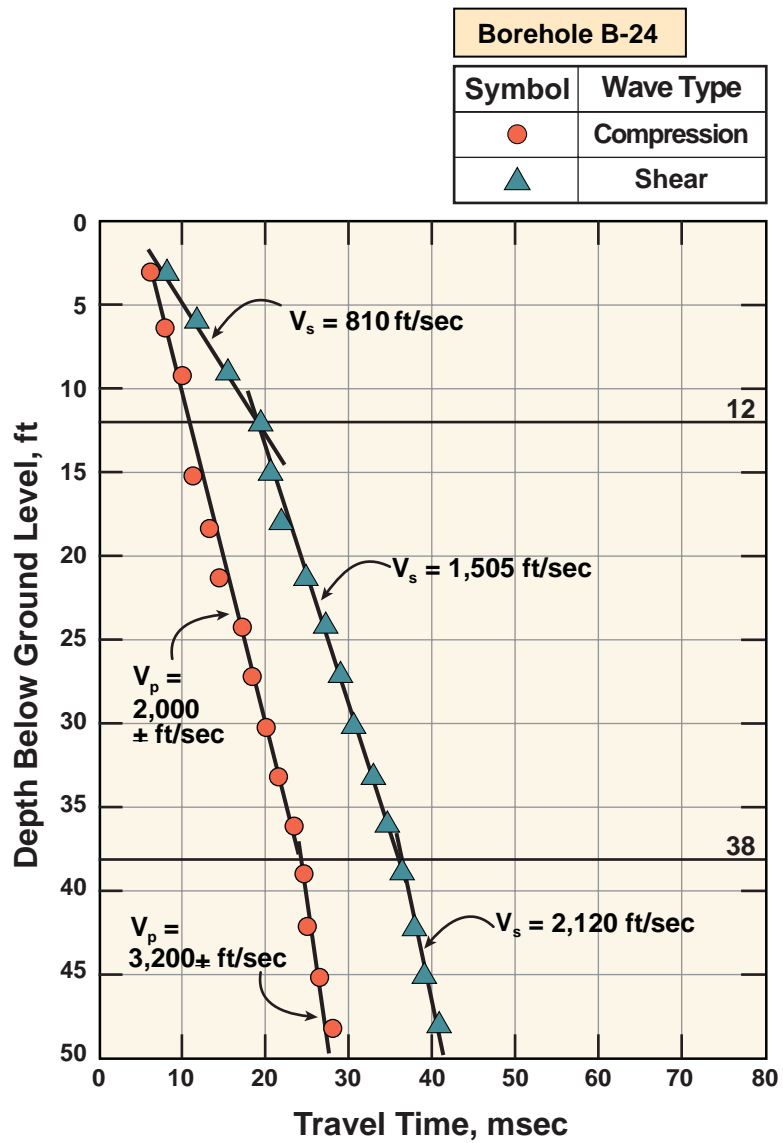
FIG_85: Results of CPT Tests in Colma Sand Illustrating Variability



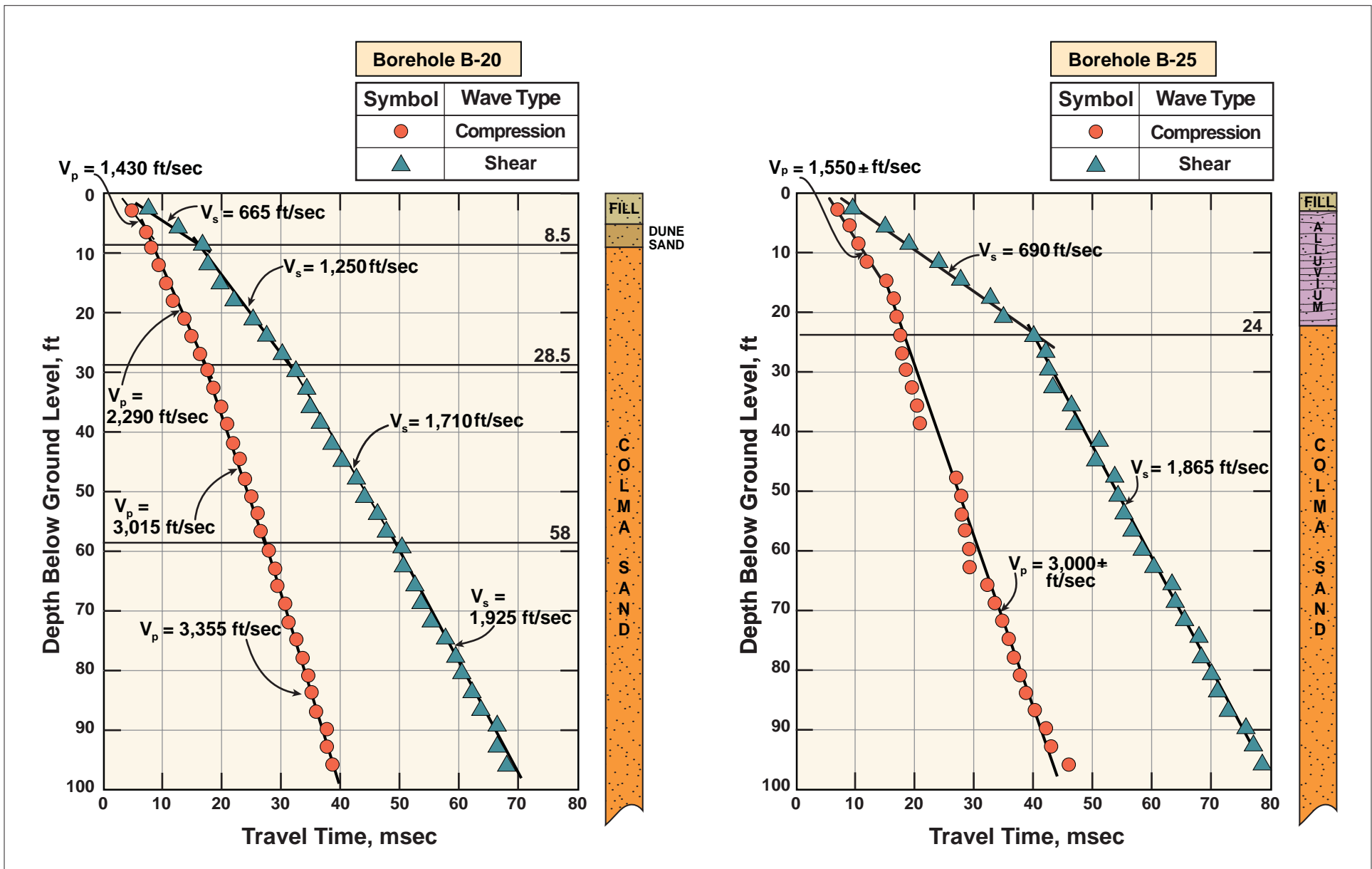
FIG_86: Shear Wave Velocities of Colma Sand Mission Bay Site



FIG_87: Shear Wave Velocities of Colma Sand Transbay Terminal Site



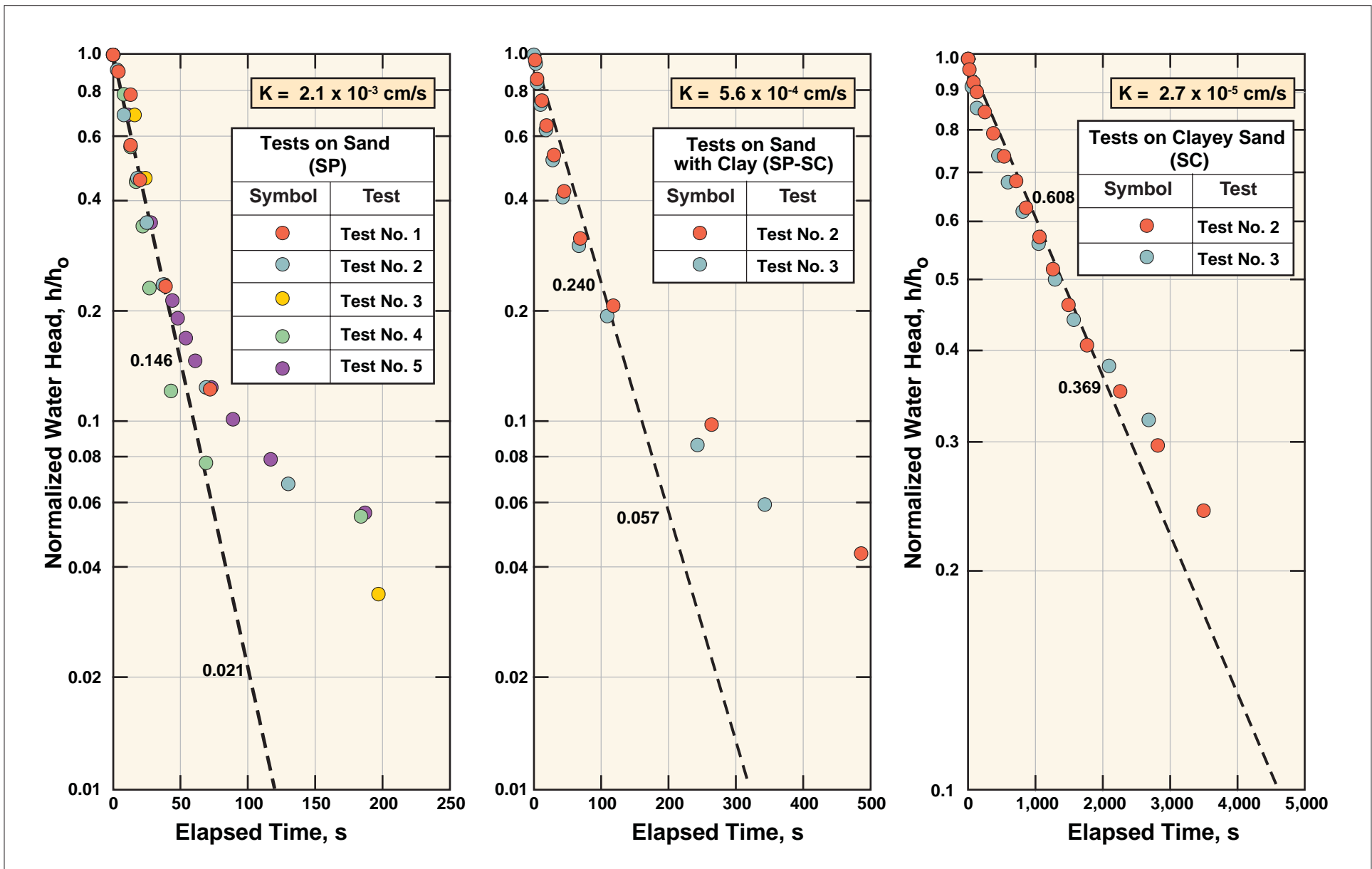
FIG_88: Shear and Compression Wave Velocities of Colma Sand Laguna Hond
 W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\FIG_88



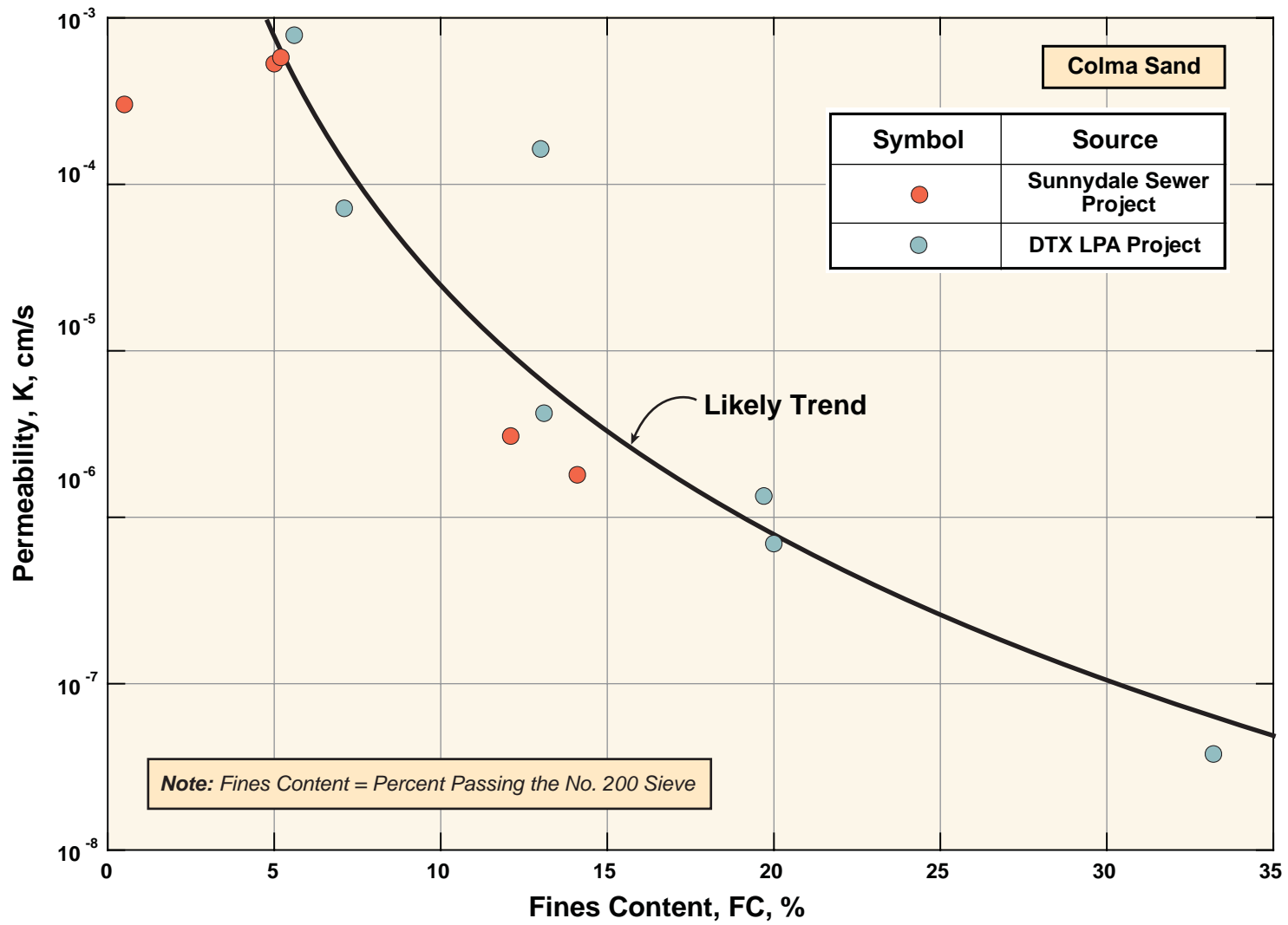
FIG_89: Shear Wave Velocities of Colma Sand Laguna Honda Hospital Site

Results of Rising-Head Permeability Tests					
Borehole ID	Screened Interval (ft)	Screen Length, (ft)	Soil Type(s)	Permeability, K	
				(ft/sec)	(cm/s)
BH-05	55 to 74	19	Colma Sand	1.64E-04	5.00E-03
FV-07	61.5 to 80	19	Colma Sand	3.55E-05	1.08E-03
B-4-6	27 to 42	15	Colma Sand, some Hard Clay	1.84E-05	5.61E-04
B-4-12	25 to 45	20	Colma Sand	3.09E-05	9.41E-04
B-4-15	17 to 37	20	Colma Sand	1.94E-05	5.90E-04
B-4-16	15 to 35	20	Colma Sand	2.49E-05	7.60E-04
B-4-17	9.5 to 19.5	10	Colam Sand	5.19E-05	1.58E-03
B-4-21	15 to 25	10	Colma Sand	6.80E-05	2.07E-03
B-4-24	15 to 25	10	Colma Sand	6.34E-05	1.93E-03
B-4-25	15 to 25	10	Colma Sand, Hard Clay	2.77E-06	8.44E-05
				7.22E-06	2.20E-04
Average				4.42E-05	1.35E-03

FIG_90: In-Situ Permeabilities of Colma Sand from Rising-Head Tests in Standpipe

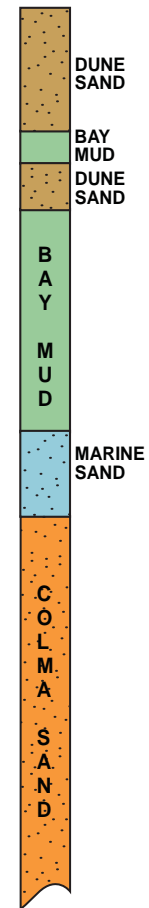
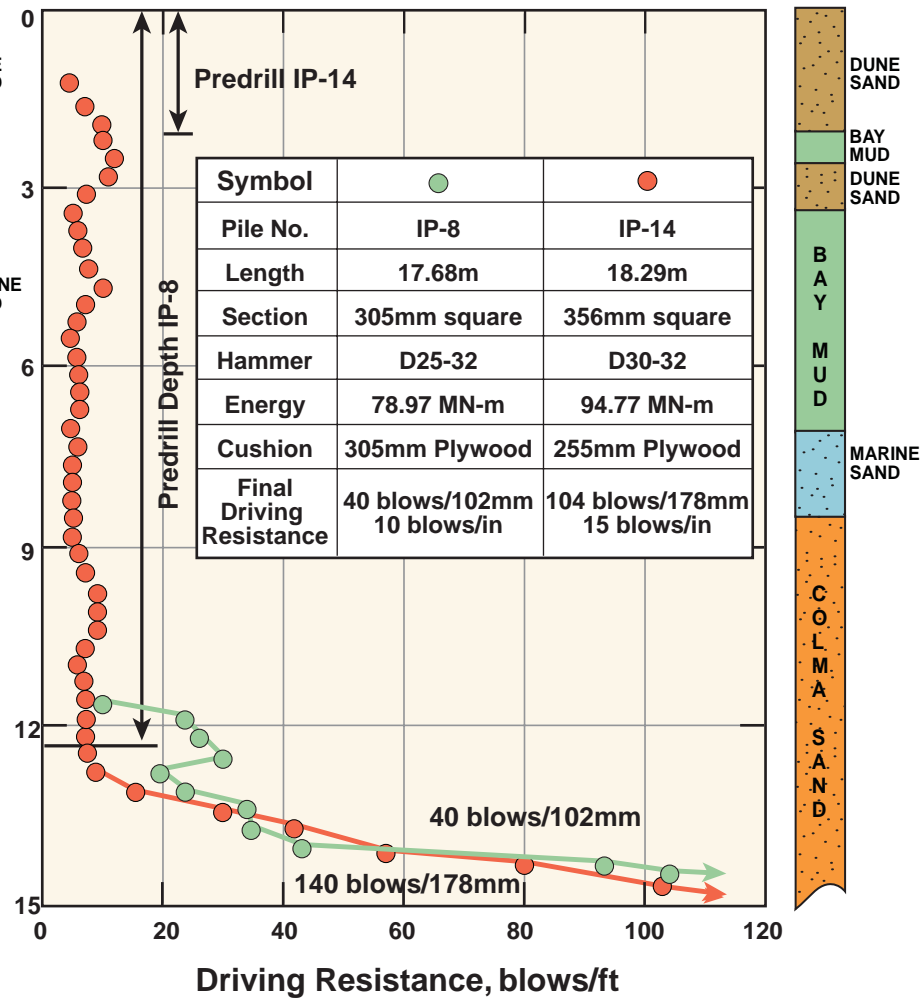
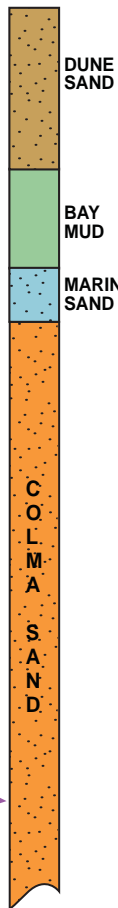
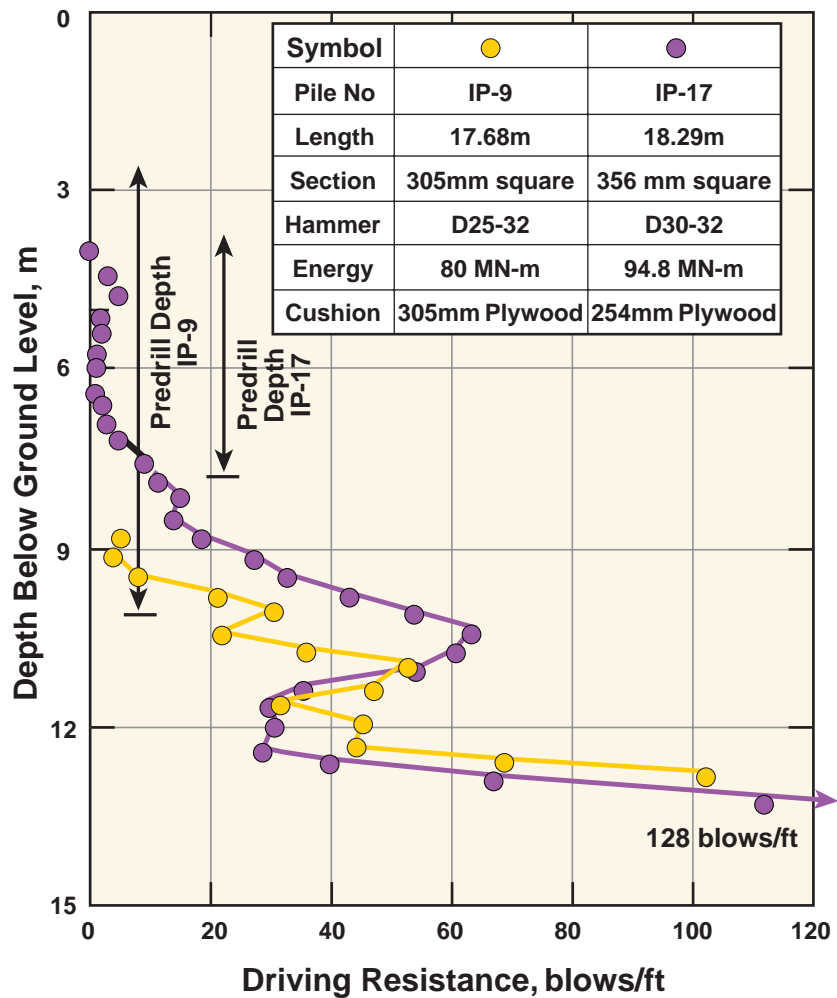


FIG_91: Typical Results of Rising-Head Permeability Tests Performed in Standpipe Piez

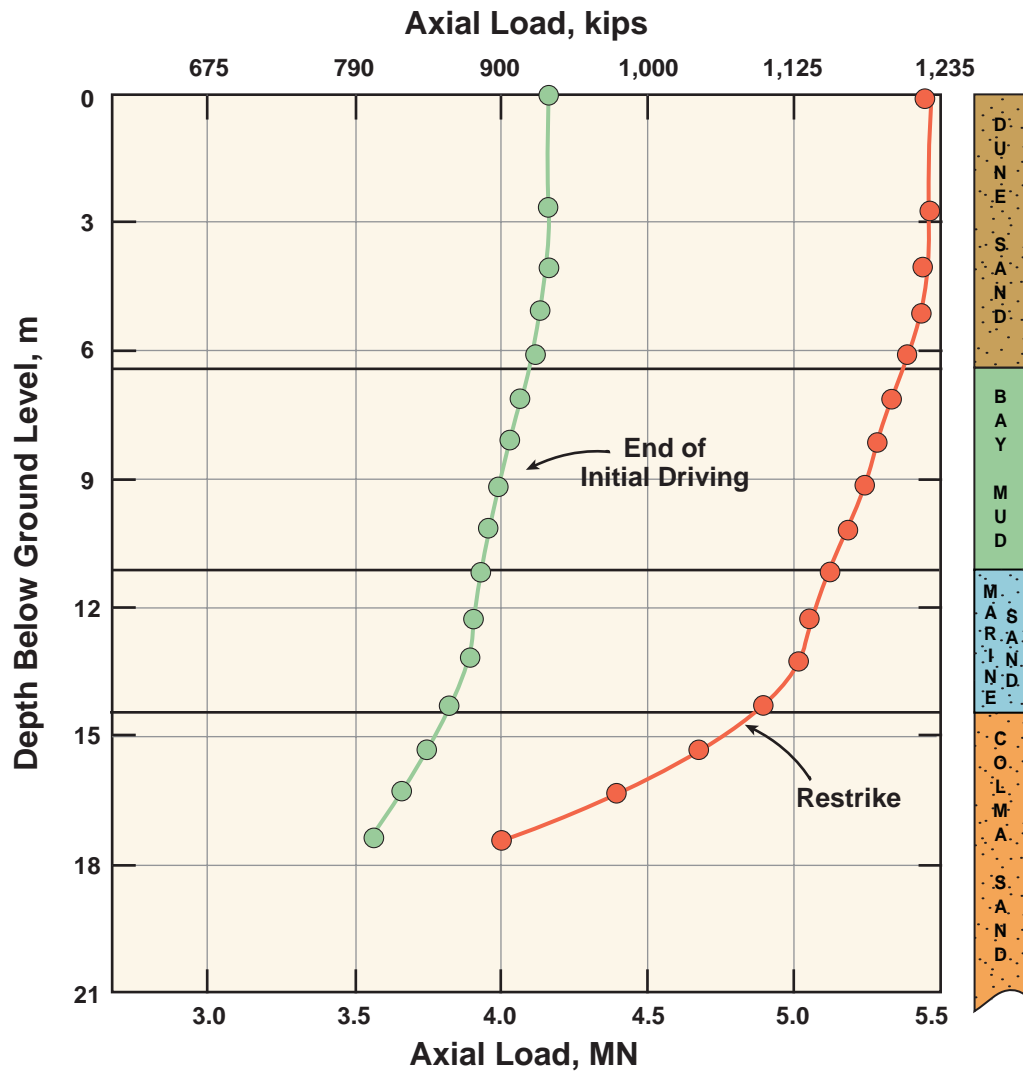


FIG_92: Variation of Vertical Permeability with Fines Content

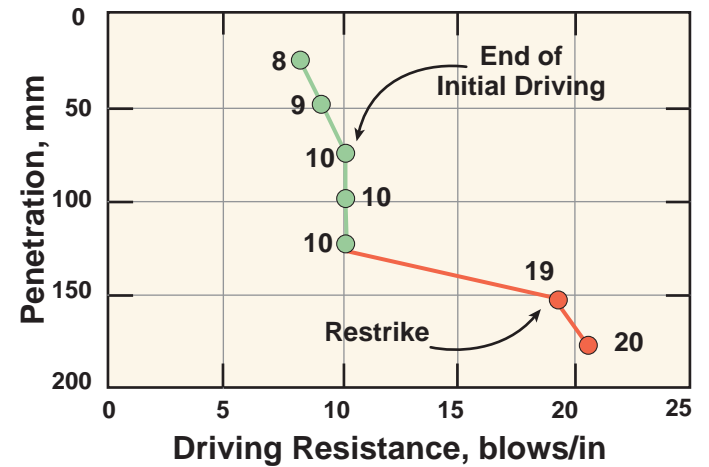
W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\04 COLMA SAND (79-103)\FIG_92



FIG_93: Typical Driving Records of Prestressed Concrete Piles Driven into Colma Sand

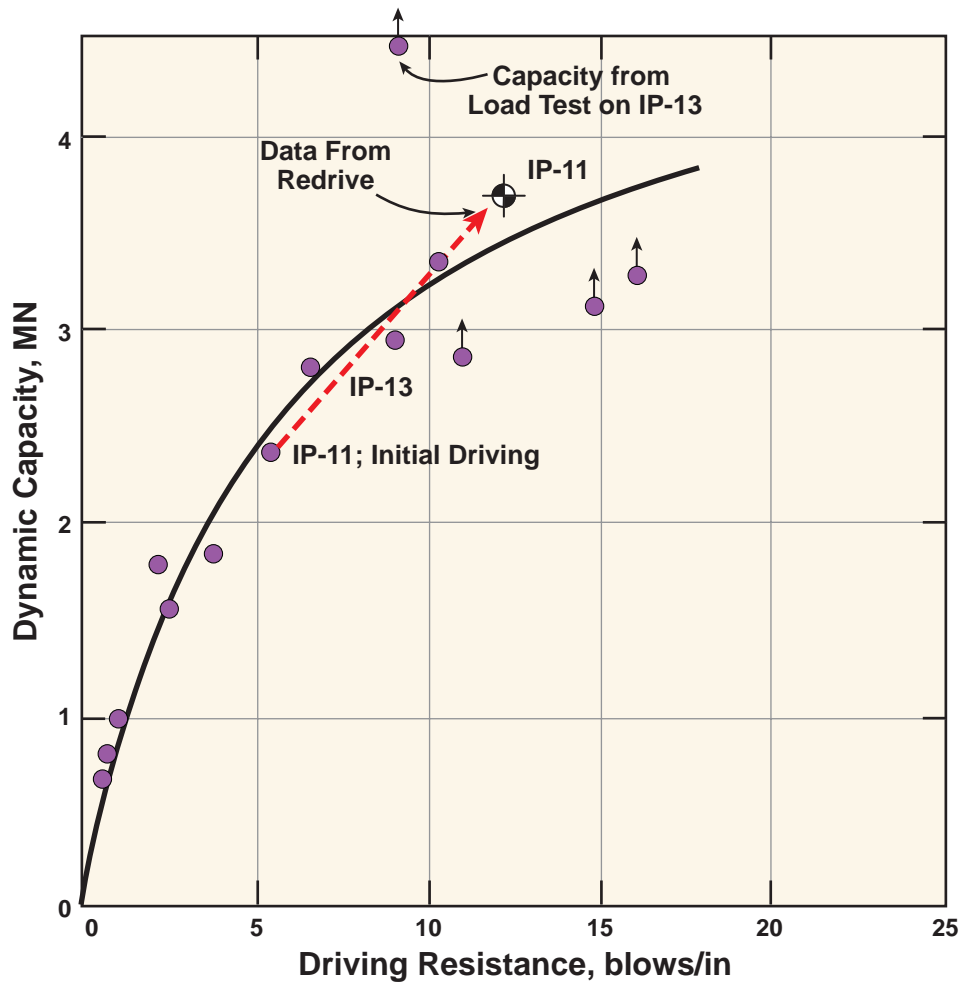


Pile No. IP-11	
Length	19.81m
Hammer	D46-32 (FS-4)
Energy	145.34 MN-m
Cushion	229mm Plywood
Rest Period	17 hr - 30 min
Pile Size	406mm (16 in)



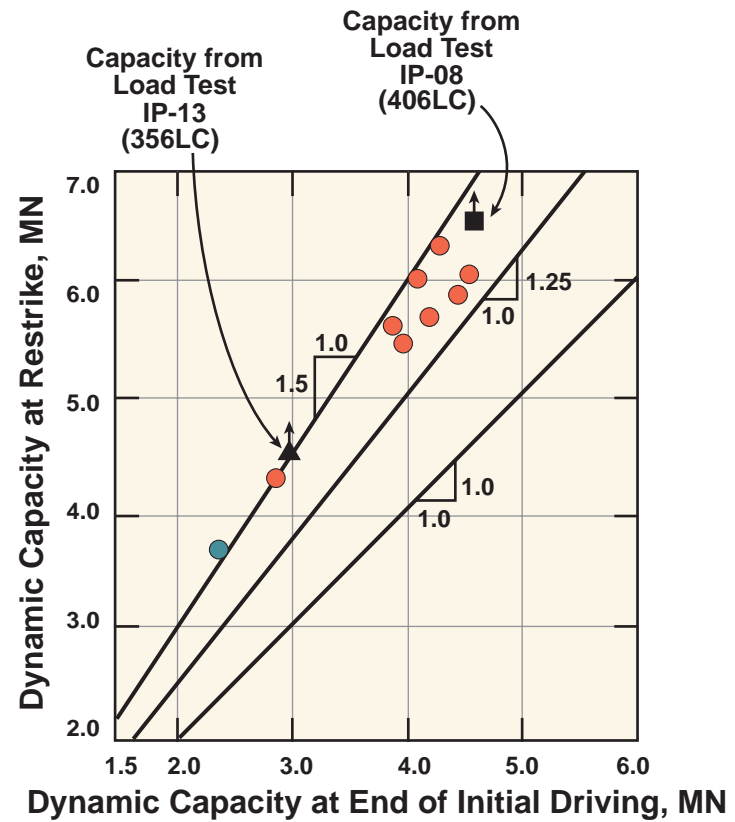
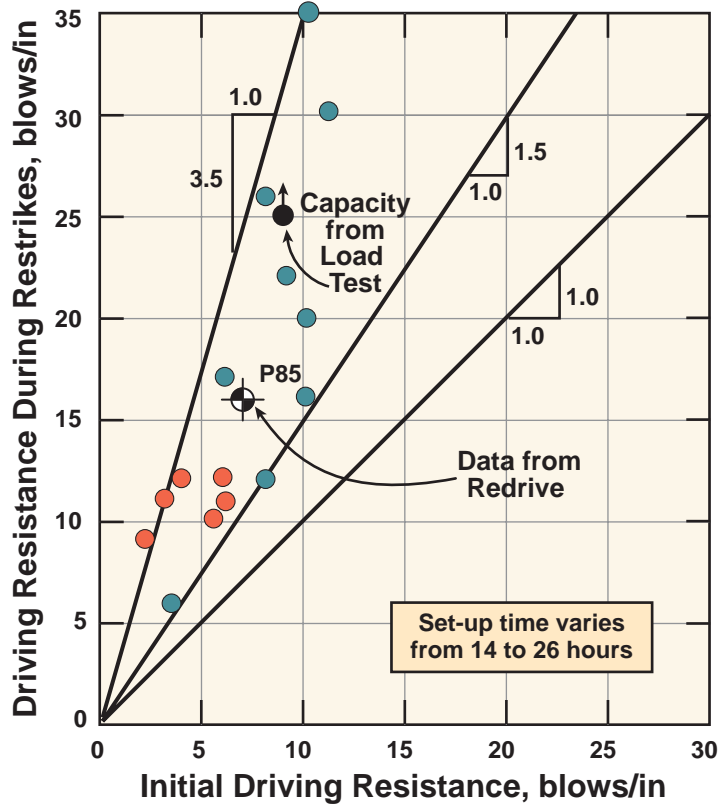
FIG_94: Results of PDA Tests on Concrete Piles Driven in Colma Sand



W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\04 COLMA SAND (79-103)\FIG_94



Pile No. IP-11 to IP-18	
Section	356mm square
Pile Lengths	18.3 to 20.1m
Hammer	D30-32
Energy	94.77 MN-m
Cushion	254mm Plywood
Measured Energy	30-41 MN-m

FIG_95 : Dynamic Capacity Versus Driving Resistance from PDA Prestressed Concrete Piles Driven into Colma Sand

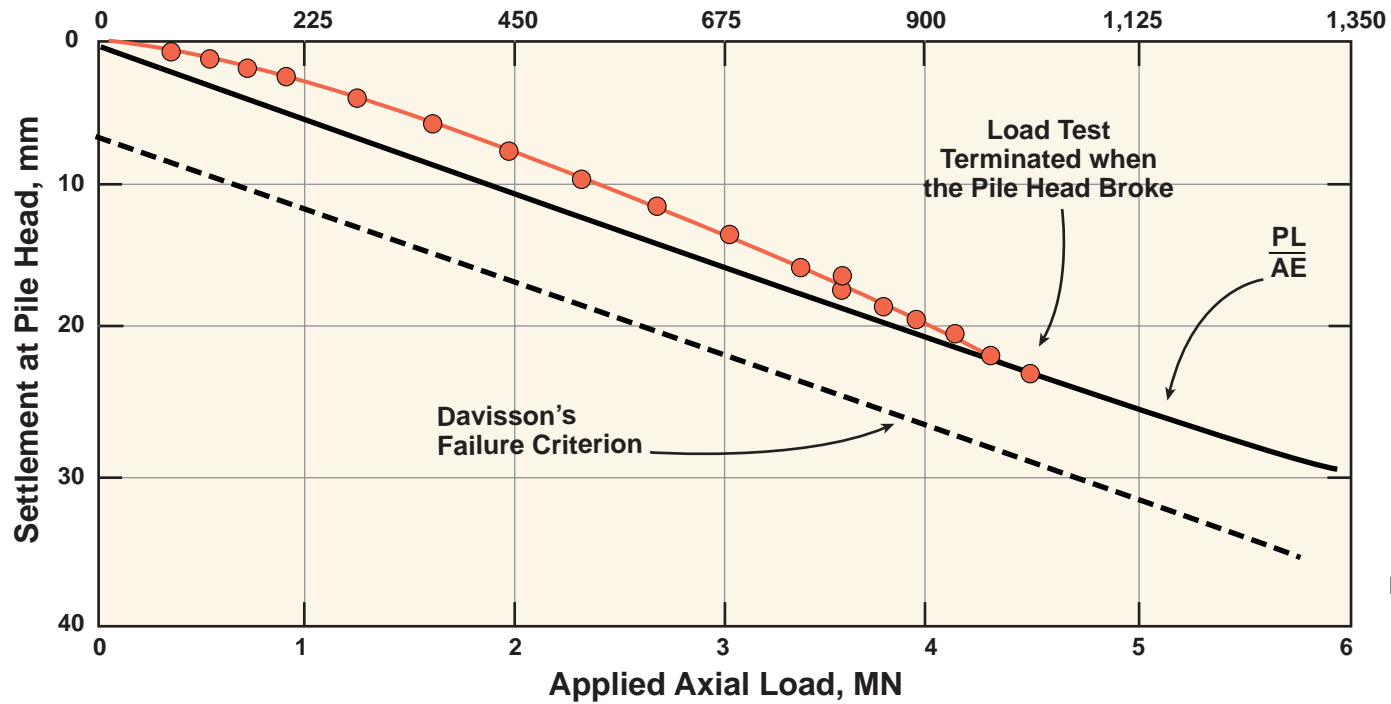


Symbol		
Pile Size	406mm (16 in)	356mm (12 in)
Hammer	D46-32 (FS-4)	D46-32 (FS-2)
Rated Energy	146 MN-M	95 MN-M

FIG_96: Effects of Soil Set-Up on Driving Resistance and Axial Capacity 12 in and 16 in Concrete Piles



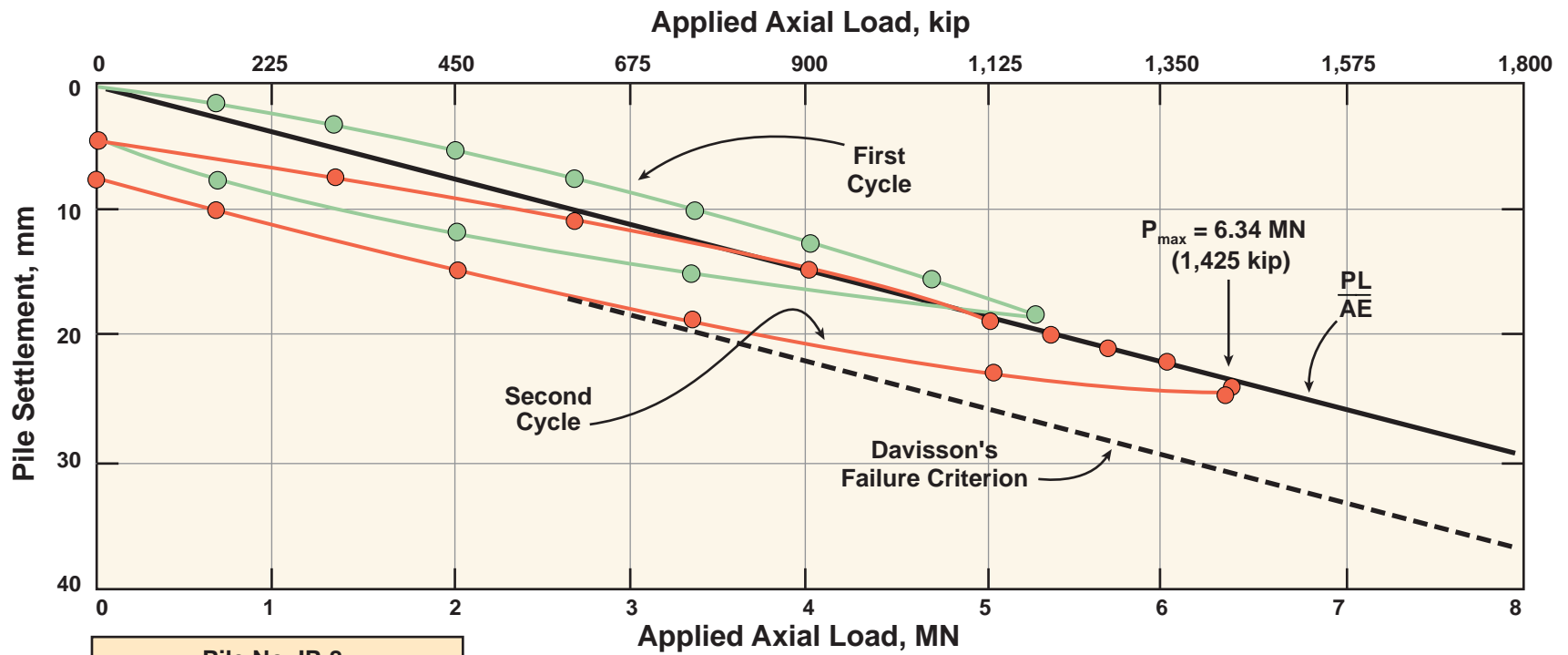
Applied Axial Load, kips



Pile No. IP-13
*14-in Square Prestressed
 Precast Concrete Pile*

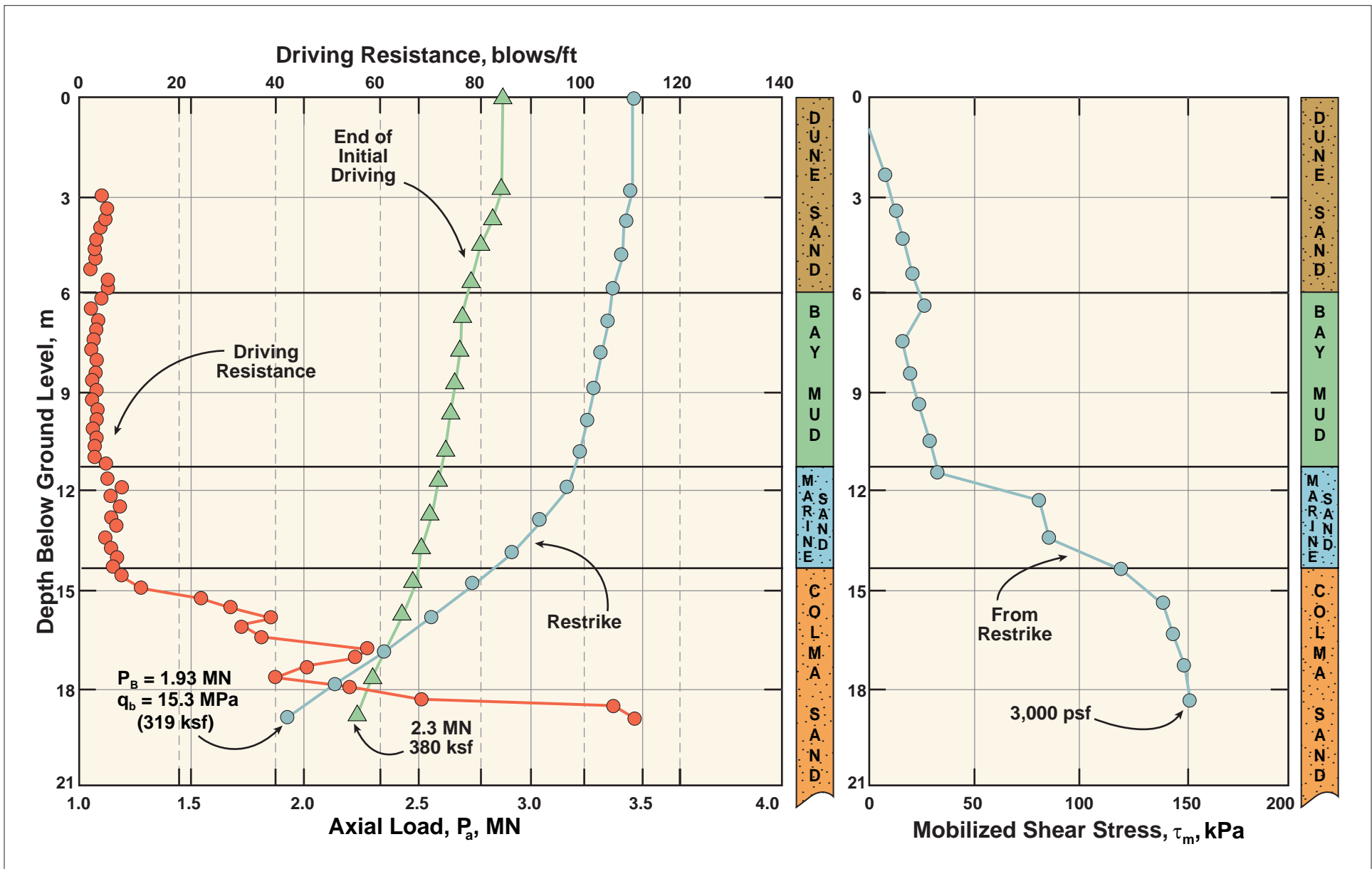
Pile Length	19.66m
Hammer	D 30-32
Rated Energy	94.77 MN-m
Cushion	254mm Plywood
Final Driving Resistance	64 blows/178mm
Penetration into Bearing Sand	3.66m

FIG_97: Results of Load Test on 14-in Prestressed Concrete Pile



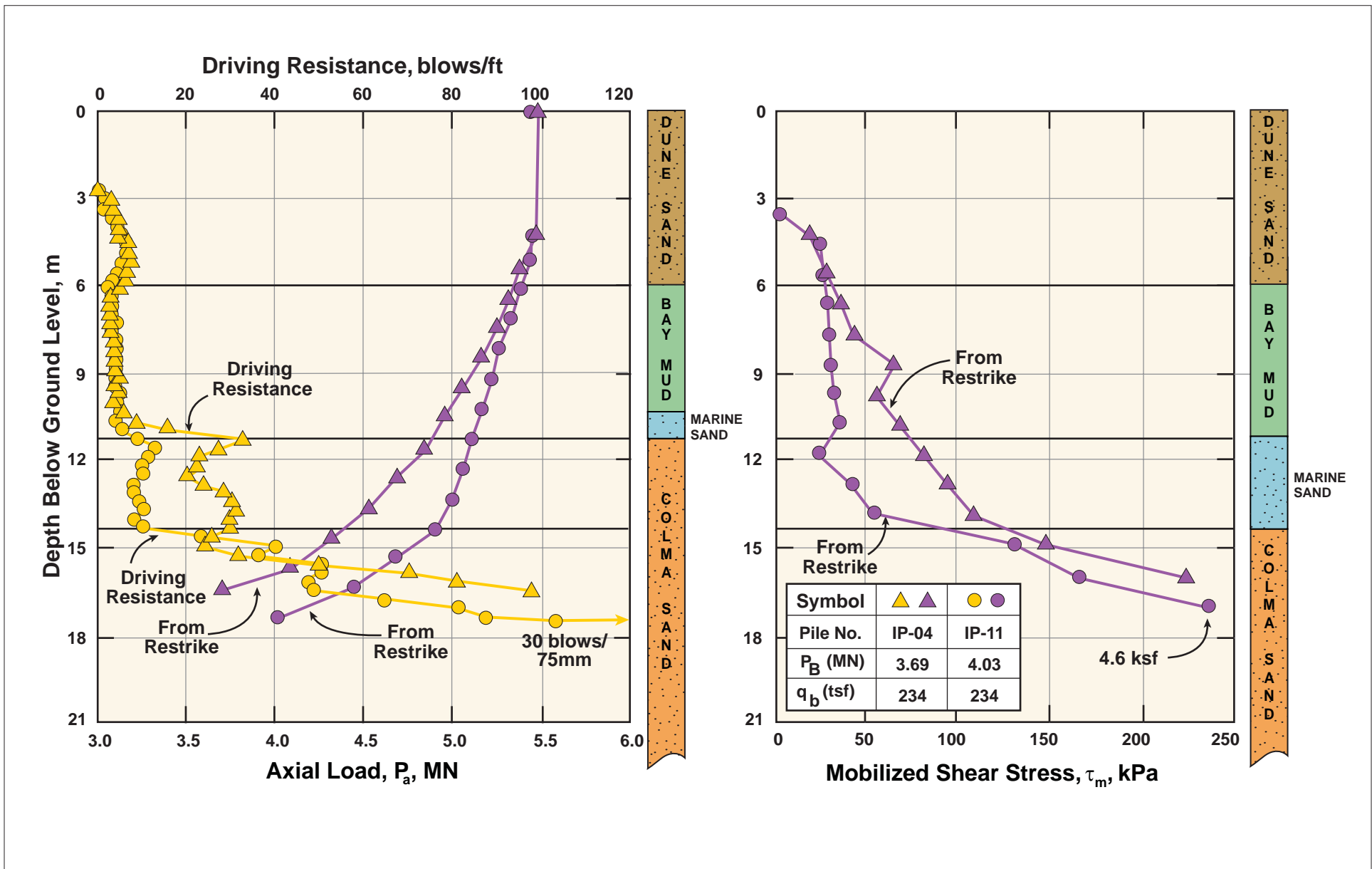
Pile No. IP-8 14-in Square Prestressed Precast Concrete Pile	
Pile Length	17.6m
Hammer	D46-32
Energy	145 MN-m
Cushion	229mm Plywood
Final Driving Resistance	7 blows/in
Penetration into Bearing Sands	6.1m

FIG_98: Results of Axial Compression Load Test 16-Inch Pre-Stressed Concrete Pile

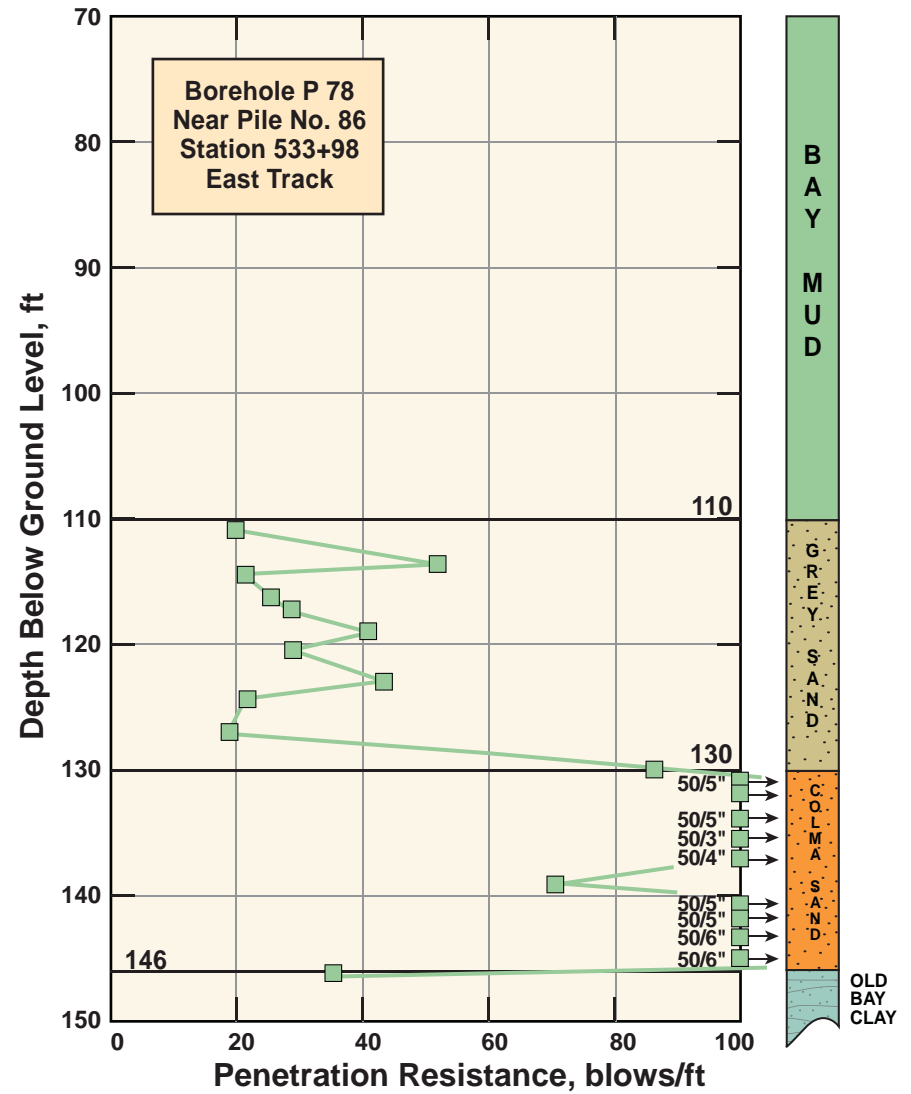
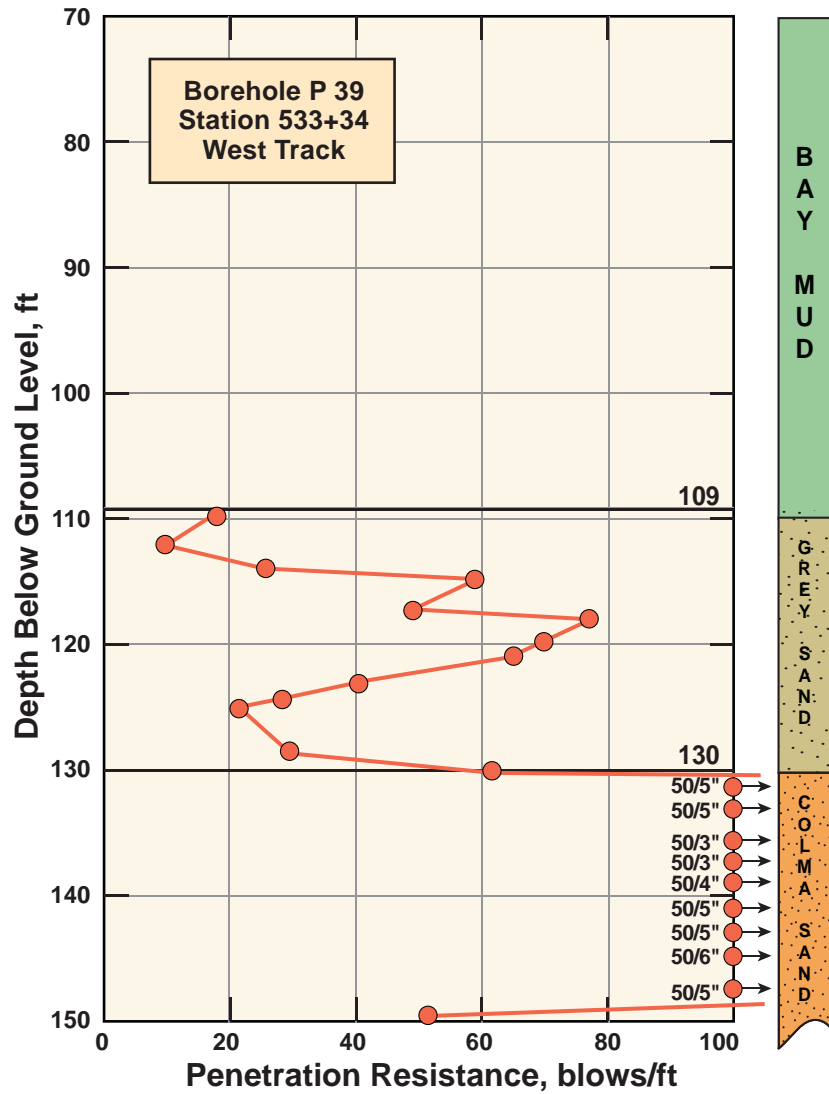


FIG_99: Axial Load Distribution, Mobilized Skin Friction, and End-Bearing Resistance from PDA Tests

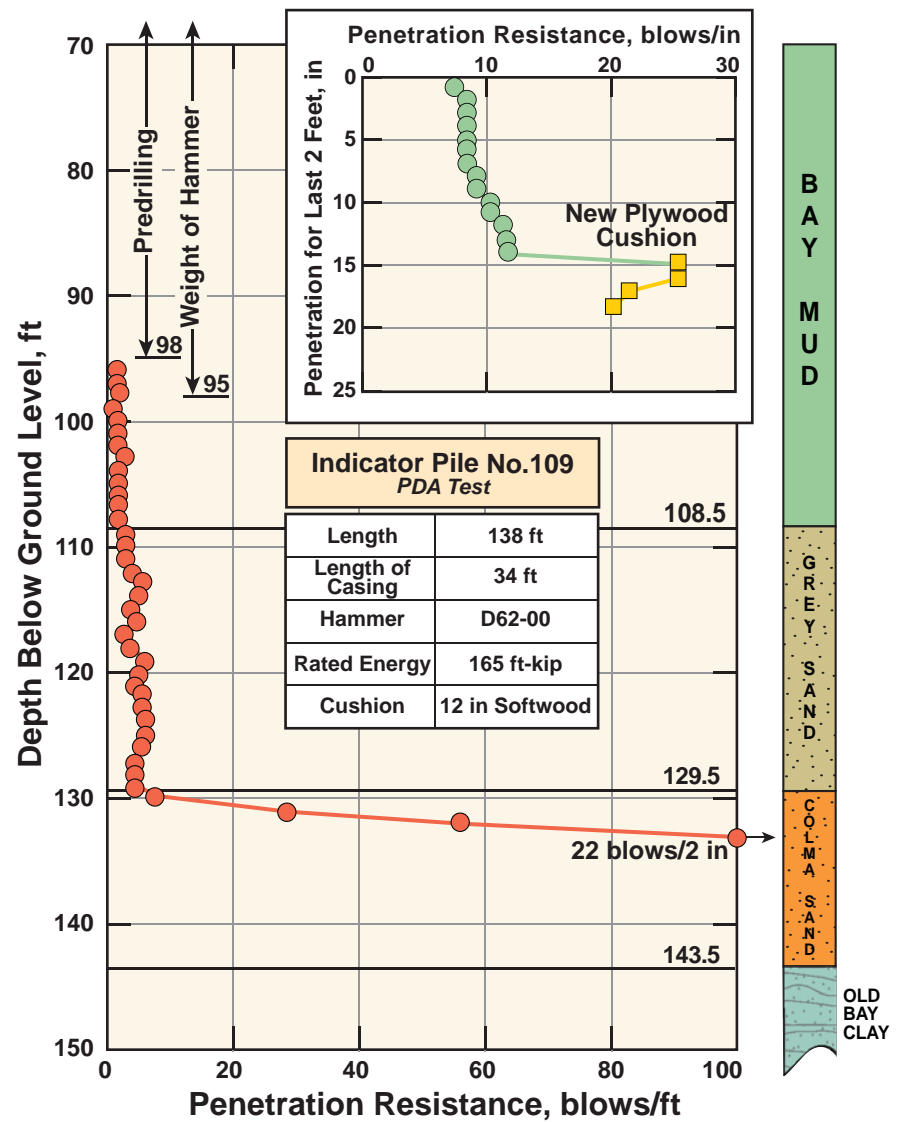
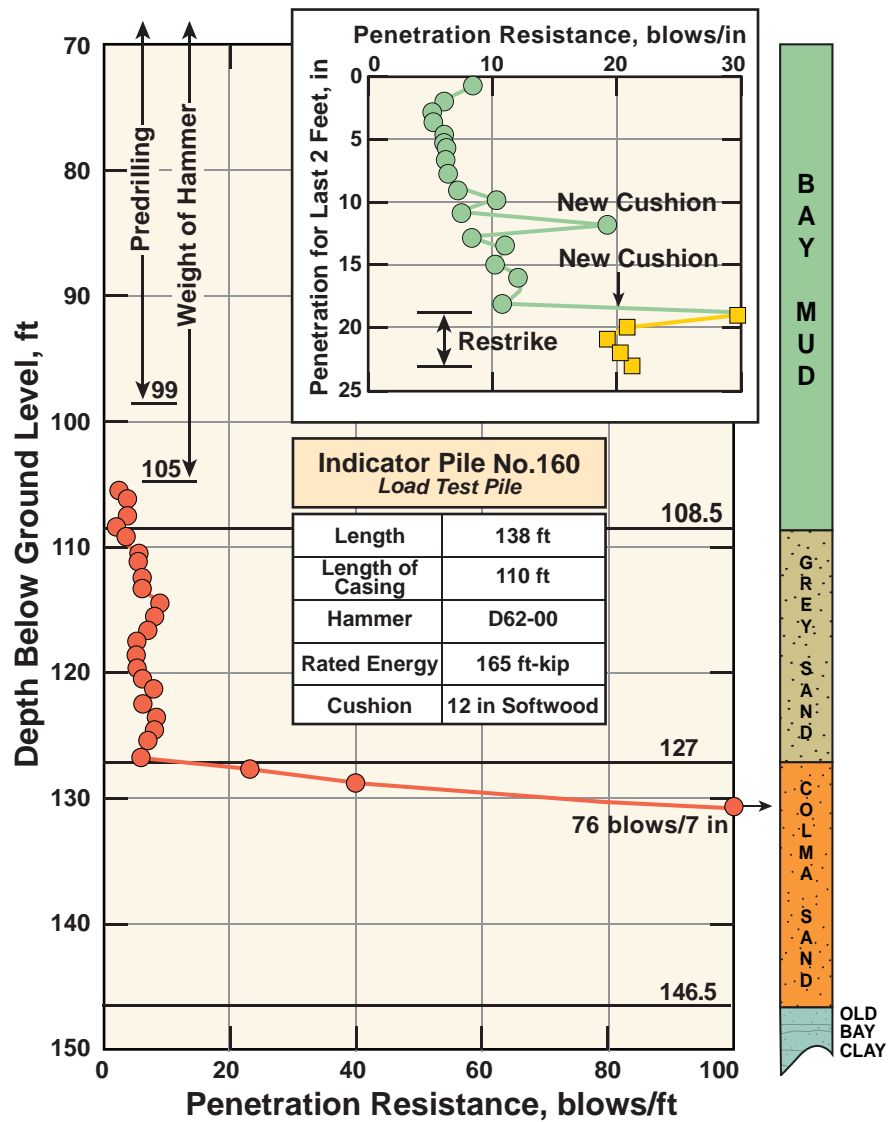
W:\Infrastructure\Geotech\UC Berkeley 2008 Seminar\Final Figures\04 COLMA SAND (79-103)\Final Figures\FIG_99



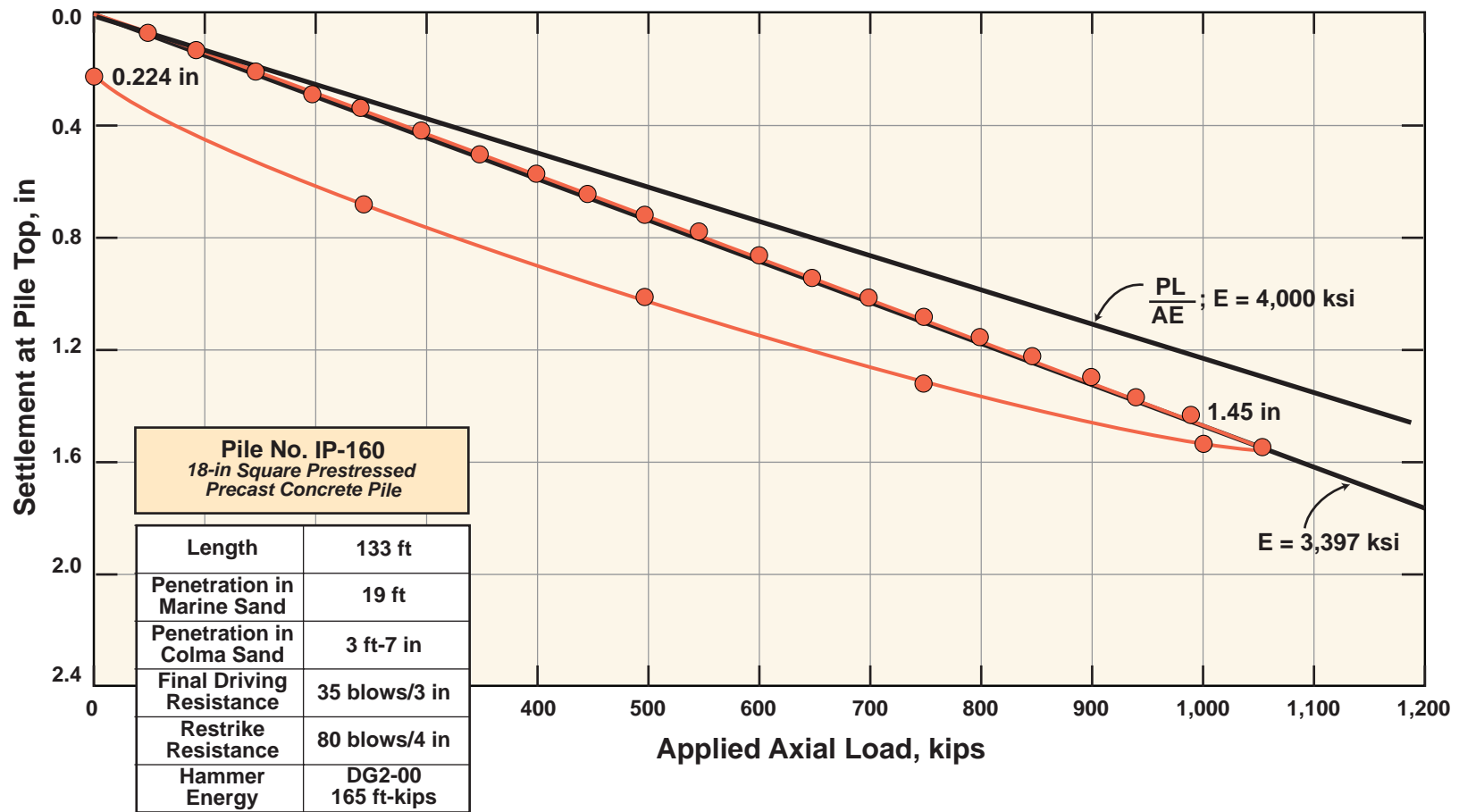
FIG_100: Axial Load Distribution, Mobilized Skin Friction, and End-Bearing Resistance from PDA Tests Site No. 2, Indicator Pile IP-11; Restrike Test



FIG_101: Standard Penetration Resistance Muni Metro Turnback Site



FIG_102: Penetration Resistance Versus Depth 18 Inch Concrete Piles Driven into Colma Sand Muni Metro Turnback Site



FIG_103: Results of Pile Load Test on 18 Inch Concrete Pile Muni Metro Turnback Site